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The Stables High Cogges, Witney Oxfordshire, OX29 6UN		
Date 14/05/2026 08:54 File Detailed design.MDX	Designed by Tim.Trotman Checked by	
Innovyze	Network 2020.1.3	

STORM SEWER DESIGN by the Modified Rational Method

Design Criteria for Storm

Pipe Sizes STANDARD Manhole Sizes STANDARD

FSR Rainfall Model - England and Wales

Return Period (years)	2	PIMP (%)	100
M5-60 (mm)	20.600	Add Flow / Climate Change (%)	0
Ratio R	0.437	Minimum Backdrop Height (m)	0.200
Maximum Rainfall (mm/hr)	50	Maximum Backdrop Height (m)	1.500
Maximum Time of Concentration (mins)	30	Min Design Depth for Optimisation (m)	1.200
Foul Sewage (l/s/ha)	0.000	Min Vel for Auto Design only (m/s)	1.00
Volumetric Runoff Coeff.	0.750	Min Slope for Optimisation (1:X)	500

Designed with Level Soffits

Time Area Diagram for Storm

Time (mins)	Area (ha)	Time (mins)	Area (ha)
0-4	0.133	4-8	0.054

Total Area Contributing (ha) = 0.187

Total Pipe Volume (m³) = 3.855

Network Design Table for Storm

« - Indicates pipe capacity < flow

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
1.000	9.000	0.113	80.0	0.011	5.00	0.0	0.600	o	100	Pipe/Conduit	
1.001	10.000	0.125	80.0	0.006	0.00	0.0	0.600	o	150	Pipe/Conduit	
1.002	10.000	0.125	80.0	0.003	0.00	0.0	0.600	o	150	Pipe/Conduit	
1.003	9.500	0.119	80.0	0.013	0.00	0.0	0.600	o	150	Pipe/Conduit	
1.004	15.500	0.194	79.9	0.025	0.00	0.0	0.600	o	150	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
1.000	50.00	5.17	30.645	0.011	0.0	0.0	0.0	0.86	6.8	1.5
1.001	50.00	5.32	30.483	0.017	0.0	0.0	0.0	1.12	19.9	2.3
1.002	50.00	5.47	30.358	0.020	0.0	0.0	0.0	1.12	19.9	2.7
1.003	50.00	5.61	30.233	0.033	0.0	0.0	0.0	1.12	19.9	4.5
1.004	50.00	5.84	30.114	0.058	0.0	0.0	0.0	1.13	19.9	7.9

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Network Design Table for Storm

PN	Length (m)	Fall (m)	Slope (1:X)	I.Area (ha)	T.E. (mins)	Base Flow (l/s)	k (mm)	HYD SECT	DIA (mm)	Section Type	Auto Design
2.000	21.500	0.215	100.0	0.027	5.00	0.0	0.600	o	150	Pipe/Conduit	
2.001	20.000	0.123	162.0	0.024	0.00	0.0	0.600	o	225	Pipe/Conduit	
2.002	3.000	0.015	200.0	0.000	0.00	0.0	0.600	o	225	Pipe/Conduit	
1.005	11.500	0.058	200.0	0.025	0.00	0.0	0.600	o	225	Pipe/Conduit	
1.006	9.500	0.048	200.0	0.012	0.00	0.0	0.600	o	225	Pipe/Conduit	
1.007	2.000	0.010	200.0	0.000	0.00	0.0	0.600	o	225	Pipe/Conduit	
1.008	10.000	0.020	500.0	0.012	0.00	0.0	0.600	o	225	Pipe/Conduit	
1.009	2.700	0.027	100.0	0.000	0.00	0.0	0.600	o	225	Pipe/Conduit	
3.000	10.600	0.177	60.0	0.008	5.00	0.0	0.600	o	100	Pipe/Conduit	
3.001	6.500	0.325	20.0	0.021	0.00	0.0	0.600	o	100	Pipe/Conduit	
1.010	8.000	0.080	100.0	0.000	0.00	0.0	0.600	o	150	Pipe/Conduit	

Network Results Table

PN	Rain (mm/hr)	T.C. (mins)	US/IL (m)	Σ I.Area (ha)	Σ Base Flow (l/s)	Foul (l/s)	Add Flow (l/s)	Vel (m/s)	Cap (l/s)	Flow (l/s)
2.000	50.00	5.36	30.273	0.027	0.0	0.0	0.0	1.00	17.8	3.7
2.001	50.00	5.68	29.983	0.051	0.0	0.0	0.0	1.02	40.7	6.9
2.002	50.00	5.74	29.860	0.051	0.0	0.0	0.0	0.92	36.6	6.9
1.005	50.00	6.05	29.845	0.134	0.0	0.0	0.0	0.92	36.6	18.1
1.006	50.00	6.22	29.787	0.146	0.0	0.0	0.0	0.92	36.6	19.8
1.007	50.00	6.26	29.740	0.146	0.0	0.0	0.0	0.92	36.6	19.8
1.008	50.00	6.55	29.730	0.158	0.0	0.0	0.0	0.58	23.0	21.4
1.009	50.00	6.58	29.710	0.158	0.0	0.0	0.0	1.31	52.0	21.4
3.000	50.00	5.18	30.184	0.008	0.0	0.0	0.0	1.00	7.8	1.1
3.001	50.00	5.24	30.007	0.029	0.0	0.0	0.0	1.73	13.6	3.9
1.010	50.00	6.71	0.000	0.187	0.0	0.0	0.0	1.00	17.8«	25.3

Free Flowing Outfall Details for Storm

Outfall Pipe Number	Outfall Name	C. Level (m)	I. Level (m)	Min I. Level (m)	D,I (mm)	W (mm)
1.010		31.390	-0.080	0.000	0	0

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<u>Simulation Criteria for Storm</u> <table> <tr> <td>Volumetric Runoff Coeff</td> <td>0.750</td> <td>Additional Flow - % of Total Flow</td> <td>0.000</td> </tr> <tr> <td>Areal Reduction Factor</td> <td>1.000</td> <td>MADD Factor * 10m³/ha Storage</td> <td>2.000</td> </tr> <tr> <td>Hot Start (mins)</td> <td>0</td> <td>Inlet Coefficient</td> <td>0.800</td> </tr> <tr> <td>Hot Start Level (mm)</td> <td>0</td> <td>Flow per Person per Day (l/per/day)</td> <td>0.000</td> </tr> <tr> <td>Manhole Headloss Coeff (Global)</td> <td>0.500</td> <td>Run Time (mins)</td> <td>60</td> </tr> <tr> <td>Foul Sewage per hectare (l/s)</td> <td>0.000</td> <td>Output Interval (mins)</td> <td>1</td> </tr> </table> <table> <tr> <td>Number of Input Hydrographs</td> <td>0</td> <td>Number of Storage Structures</td> <td>2</td> </tr> <tr> <td>Number of Online Controls</td> <td>1</td> <td>Number of Time/Area Diagrams</td> <td>0</td> </tr> <tr> <td>Number of Offline Controls</td> <td>0</td> <td>Number of Real Time Controls</td> <td>0</td> </tr> </table> <u>Synthetic Rainfall Details</u> <table> <tr> <td>Rainfall Model</td> <td>FSR</td> <td>Profile Type</td> <td>Summer</td> </tr> <tr> <td>Return Period (years)</td> <td>2</td> <td>Cv (Summer)</td> <td>0.750</td> </tr> <tr> <td>Region</td> <td>England and Wales</td> <td>Cv (Winter)</td> <td>0.840</td> </tr> <tr> <td>M5-60 (mm)</td> <td>20.600</td> <td>Storm Duration (mins)</td> <td>30</td> </tr> <tr> <td>Ratio R</td> <td>0.437</td> <td></td> <td></td> </tr> </table>			Volumetric Runoff Coeff	0.750	Additional Flow - % of Total Flow	0.000	Areal Reduction Factor	1.000	MADD Factor * 10m³/ha Storage	2.000	Hot Start (mins)	0	Inlet Coefficient	0.800	Hot Start Level (mm)	0	Flow per Person per Day (l/per/day)	0.000	Manhole Headloss Coeff (Global)	0.500	Run Time (mins)	60	Foul Sewage per hectare (l/s)	0.000	Output Interval (mins)	1	Number of Input Hydrographs	0	Number of Storage Structures	2	Number of Online Controls	1	Number of Time/Area Diagrams	0	Number of Offline Controls	0	Number of Real Time Controls	0	Rainfall Model	FSR	Profile Type	Summer	Return Period (years)	2	Cv (Summer)	0.750	Region	England and Wales	Cv (Winter)	0.840	M5-60 (mm)	20.600	Storm Duration (mins)	30	Ratio R	0.437		
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Online Controls for Storm Hydro-Brake® Optimum Manhole: 16, DS/PN: 1.010, Volume (m³): 35.6 <table><tr><td>Unit Reference</td><td>MD-SHE-0078-2600-0900-2600</td></tr><tr><td>Design Head (m)</td><td>0.900</td></tr><tr><td>Design Flow (l/s)</td><td>2.6</td></tr><tr><td>Flush-Flo™</td><td>Calculated</td></tr><tr><td>Objective</td><td>Minimise upstream storage</td></tr><tr><td>Application</td><td>Surface</td></tr><tr><td>Sump Available</td><td>Yes</td></tr><tr><td>Diameter (mm)</td><td>78</td></tr><tr><td>Invert Level (m)</td><td>29.682</td></tr><tr><td>Minimum Outlet Pipe Diameter (mm)</td><td>100</td></tr><tr><td>Suggested Manhole Diameter (mm)</td><td>1200</td></tr></table> <table><tr><td>Control Points</td><td>Head (m)</td><td>Flow (l/s)</td></tr><tr><td>Design Point (Calculated)</td><td>0.900</td><td>2.6</td></tr><tr><td>Flush-Flo™</td><td>0.271</td><td>2.6</td></tr><tr><td>Kick-Flo®</td><td>0.569</td><td>2.1</td></tr><tr><td>Mean Flow over Head Range</td><td>-</td><td>2.3</td></tr></table> The hydrological calculations have been based on the Head/Discharge relationship for the Hydro-Brake® Optimum as specified. Should another type of control device other than a Hydro-Brake Optimum® be utilised then these storage routing calculations will be invalidated <table><tr><td>Depth (m)</td><td>Flow (l/s)</td><td>Depth (m)</td><td>Flow (l/s)</td><td>Depth (m)</td><td>Flow (l/s)</td><td>Depth (m)</td><td>Flow (l/s)</td></tr><tr><td>0.100</td><td>2.2</td><td>1.200</td><td>3.0</td><td>3.000</td><td>4.5</td><td>7.000</td><td>6.8</td></tr><tr><td>0.200</td><td>2.6</td><td>1.400</td><td>3.2</td><td>3.500</td><td>4.9</td><td>7.500</td><td>7.0</td></tr><tr><td>0.300</td><td>2.6</td><td>1.600</td><td>3.4</td><td>4.000</td><td>5.2</td><td>8.000</td><td>7.2</td></tr><tr><td>0.400</td><td>2.5</td><td>1.800</td><td>3.6</td><td>4.500</td><td>5.5</td><td>8.500</td><td>7.4</td></tr><tr><td>0.500</td><td>2.4</td><td>2.000</td><td>3.8</td><td>5.000</td><td>5.8</td><td>9.000</td><td>7.6</td></tr><tr><td>0.600</td><td>2.2</td><td>2.200</td><td>3.9</td><td>5.500</td><td>6.0</td><td>9.500</td><td>7.8</td></tr><tr><td>0.800</td><td>2.5</td><td>2.400</td><td>4.1</td><td>6.000</td><td>6.3</td><td></td><td></td></tr><tr><td>1.000</td><td>2.7</td><td>2.600</td><td>4.2</td><td>6.500</td><td>6.5</td><td></td><td></td></tr></table>			Unit Reference	MD-SHE-0078-2600-0900-2600	Design Head (m)	0.900	Design Flow (l/s)	2.6	Flush-Flo™	Calculated	Objective	Minimise upstream storage	Application	Surface	Sump Available	Yes	Diameter (mm)	78	Invert Level (m)	29.682	Minimum Outlet Pipe Diameter (mm)	100	Suggested Manhole Diameter (mm)	1200	Control Points	Head (m)	Flow (l/s)	Design Point (Calculated)	0.900	2.6	Flush-Flo™	0.271	2.6	Kick-Flo®	0.569	2.1	Mean Flow over Head Range	-	2.3	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	Depth (m)	Flow (l/s)	0.100	2.2	1.200	3.0	3.000	4.5	7.000	6.8	0.200	2.6	1.400	3.2	3.500	4.9	7.500	7.0	0.300	2.6	1.600	3.4	4.000	5.2	8.000	7.2	0.400	2.5	1.800	3.6	4.500	5.5	8.500	7.4	0.500	2.4	2.000	3.8	5.000	5.8	9.000	7.6	0.600	2.2	2.200	3.9	5.500	6.0	9.500	7.8	0.800	2.5	2.400	4.1	6.000	6.3			1.000	2.7	2.600	4.2	6.500	6.5		
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<u>Summary of Critical Results by Maximum Level (Rank 1) for Storm</u>									
PN	US/MH Name	Surcharged Depth (m)	Flooded Volume (m³)	Flow / Cap.	Overflow (l/s)	Half Drain Time (mins)	Pipe Flow (l/s)	Status	Level Exceeded
1.000	1	0.215	0.000	1.00			6.3	SURCHARGED	
1.001	2	0.228	0.000	0.56			9.8	SURCHARGED	
1.002	3	0.323	0.000	0.70			12.4	SURCHARGED	
1.003	4	0.410	0.000	1.02			17.9	SURCHARGED	
1.004	5	0.422	0.000	1.67			30.7	SURCHARGED	
2.000	6	0.031	0.000	1.07			18.0	SURCHARGED	
2.001	7	0.143	0.000	0.12			4.4	SURCHARGED	
2.002	8	0.265	0.000	0.11		275	2.9	SURCHARGED	
1.005	9	0.279	0.000	0.29			9.0	SURCHARGED	
1.006	10	0.335	0.000	0.33			9.9	SURCHARGED	
1.007	11	0.380	0.000	0.34			9.7	SURCHARGED	
1.008	12	0.388	0.000	0.71			10.6	SURCHARGED	
1.009	13	0.406	0.000	0.31		338	9.3	SURCHARGED	
3.000	14	0.224	0.000	0.80			5.8	SURCHARGED	
3.001	15	0.321	0.000	1.40			17.2	SURCHARGED	
1.010	16	30.199	0.000	0.17			2.6	SURCHARGED	

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