

25th April 2024

Our reference: 890564-R (1)

London Borough of Hillingdon
Planning Services
3N/04
Civic Centre
High Street
Uxbridge, Middlesex
UB8 1UW

Dear Sirs/Madam

LIDL Foodstore Victoria Road Ruislip

The demolition of a non-food retail unit and reconfiguration of the existing car park.

A drainage statement has been produced to provide the necessary information for the Local Planning Authority, London Borough of Hillingdon, in support for a planning application for the development listed above.

The information provided in this letter and opinions expressed are subject to RSK Group Service Constraints contained in **Appendix A**.

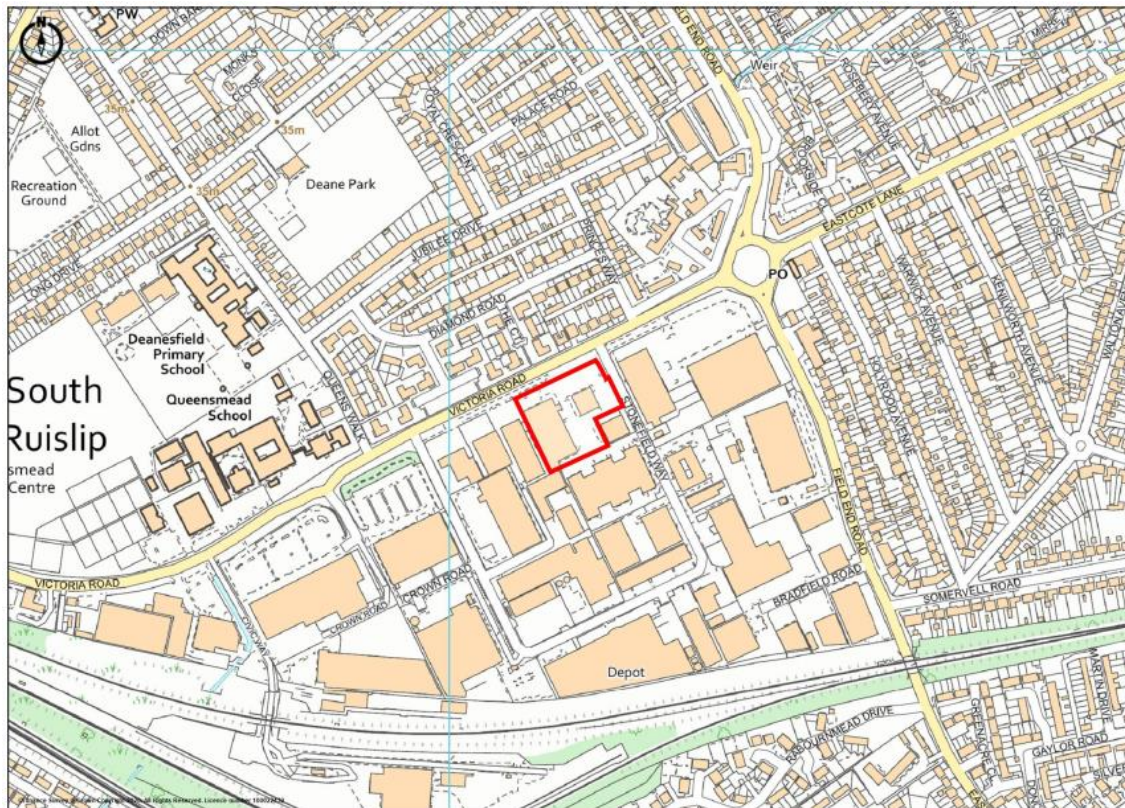
1. SITE LOCATION AND PROPOSALS

The site is located to the south of Victoria Road and to the west of Stonefield Way in South Ruislip in the London Borough of Hillingdon. Beyond the southern and western site boundaries lie other commercial properties within the Victoria Retail Park. The site can be located at post code HA4 0QQ and at National Grid Reference 512160,185588. A site location plan is included overleaf as **Figure 1**.

The site is approximately 0.94ha in size and currently comprises the Lidl South Ruislip retail store, the Benson for Beds unit and associated car parking. The existing site layout along with the topographical survey is included in **Appendix B**.

Proposals include the demolition of the Bensons for Bed unit, to provide additional car parking spaces. The proposed new site layout is included in **Appendix C**.

Figure 1: Site Location Plan



2. DEVELOPMENT DRAINAGE

2.1 EXISTING DRAINAGE

Sewer records have been obtained from Thames Water (**Appendix D**). The plans indicate the following network of public sewers in and around the site:

- A 225mm diameter foul water sewer crosses the southern portion of the site, conveying flow in a south westerly direction;
- A 225mm diameter foul water sewer is present beneath Stonefield Way and discharges into the aforementioned foul sewer;
- A 225mm diameter foul water sewer is present beneath Victoria Road, conveying flow in a south westerly direction;
- The 1219mm diameter Harrow Branch Sewer (foul trunk sewer) is located beneath Victoria Road and also conveys flow in a south westerly direction;
- The sewer records show the culverted watercourse beneath Victoria Road but indicate that this is not operated or maintained by TW;
- An 825mm diameter surface water sewer is present beneath Stonefield Way, conveying flow northwards before discharging into the culverted watercourse; and
- A number of other sewers are present surrounding the site to serve the wider area. All surface water sewers within the vicinity of the site are discharging to the culverted watercourse.

The existing Lidl store and associated car parking area have their own private drainage system. The private surface water drainage collects and attenuates the site's runoff in a geocellular tank prior to releasing it at a controlled rate of 11l/s into the culvert located on Victoria Road.

Appendix E includes a plan of the existing drainage arrangement.

2.2 OFF-SITE DISCHARGE OPTIONS

The Policy 5.13 Sustainable Drainage of the London Plan states that development proposals should aim to achieve greenfield run-off rates and ensure that surface water run-off is managed as close to its source as possible in line with the following drainage hierarchy:

- 1) Rainwater harvesting (including a combination of green and blue roofs)
- 2) Infiltration techniques and green roofs
- 3) Rainwater attenuation in open water features for gradual release
- 4) Rainwater discharge direct to a watercourse (unless not appropriate)
- 5) Rainwater attenuation above ground (including blue roofs)
- 6) Rainwater attenuation below ground
- 7) Rainwater discharge to a surface water sewer or drain
- 8) Rainwater discharge to a combined sewer

The London Plan advocates that infiltration should be considered as the primary option to discharge surface water from a developed site. However, the effectiveness of infiltration is completely dependent on the physical conditions at the site. The site is directly underlain by impermeable London Clay therefore infiltration is not considered feasible.

Discharging surface water directly to a local watercourse is not considered feasible as there are no suitable watercourses within the immediate vicinity of the site.

Following option is to discharge the surface water runoff resultant from the car park extension to the existing private surface water drainage sewer serving the Site.

2.3 PROPOSED SURFACE WATER DRAINAGE SCHEME

The limited space for development makes it impractical to use above ground SuDS elements to control the runoff in the new parking bays, Instead, gullies will be placed at the lowest points to collect the runoff.

A vortex flow control unit will be set up to replace the existing control unit to ensure that surface water discharge offsite does not exceed the existing rate of 11l/s for any rainfall events.

The existing drainage infrastructure will be retained as depicted in the surface water drainage strategy drawing in **Appendix F**.

The Hillingdon Council's section on SuDS information for major applications suggests that commercial premises should be provided with surface water storage up to the 1in100 year event plus an additional 20% to account for climate change.

To increase the capacity of the existing drainage network, a new attenuation tank will be installed at the previous location of the demolished Benson building.

Appendix G contains the drainage calculations showing that the storage volume of the existing and proposed drainage infrastructure is sufficient to accommodate the 1in100 year plus 20% climate change event, as advised.

A summary of the proposals is listed within the SuDS Proforma attached in **Appendix H**.

2.4 WATER QUALITY

The surface water runoff resulting from the future car park extension will be treated at the existing oil bypass separator installed just upstream to the existing geocellular tank. The existing unit currently has capacity to treat the additional flows resulting from the car park extension area.

2.5 MAINTENANCE AND OPERATIONAL CONSIDERATIONS

All SuDS and proprietary treatment features that will be put in place will stay under private ownership. The responsibility for inspection and maintenance will fall on either the future landowner or private management company.

Appendix I includes a comprehensive SuDS management strategy for the proposed development.

2.6 EXCEEDANCE FLOWS

The drainage strategy plan in **Appendix F** illustrates the routes for exceedance flows. There are no proposed control measures for these exceedance flows, as all surface water runoff from storm events up to the 1in100 year plus climate change will be accommodated on site.

3. CONCLUSION

This drainage statement and the associated Drainage Strategy demonstrate that the proposed development drainage complies with the requirements outlined by London Borough of Hillingdon Local Flood Authority with the main principles listed below:

- Policy SI13 Sustainable Drainage of the London Plan has been followed
- Surface water runoff treatment is provided via existing oil bypass separator complying with Policy DMEI 10: Water Management, Efficiency, and Quality in regard to avoiding pollution of the water environment.
- Surface water discharge from the site is no greater than the existing 11(l/s) for all storm period events.
- The proposed surface water drainage system caters for 20% allowance in increase intensity due to climate change with no flooding events registered
- Maintenance of drainage infrastructure to be provided by the current landowner and/or private management company
- The development proposals reduce the existing flood risk and does not increase flood risk on or offsite.

For RSK Company Limited



Assif M Salim

Enclosed:

Appendix A- RSK Group Service Constraints

Appendix B – Existing Site Plan / Topographical Survey

Appendix C – Proposed Site Plan

Appendix D – Sewer Records

Appendix E – Existing Drainage

Appendix F – Proposed Surface Water Drainage Strategy

Appendix G – Surface Water Drainage Calculations

Appendix H – SuDS Proforma

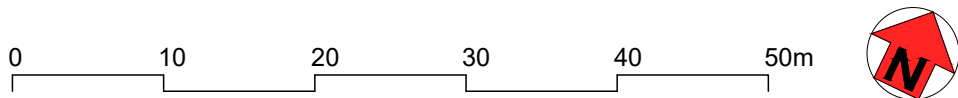
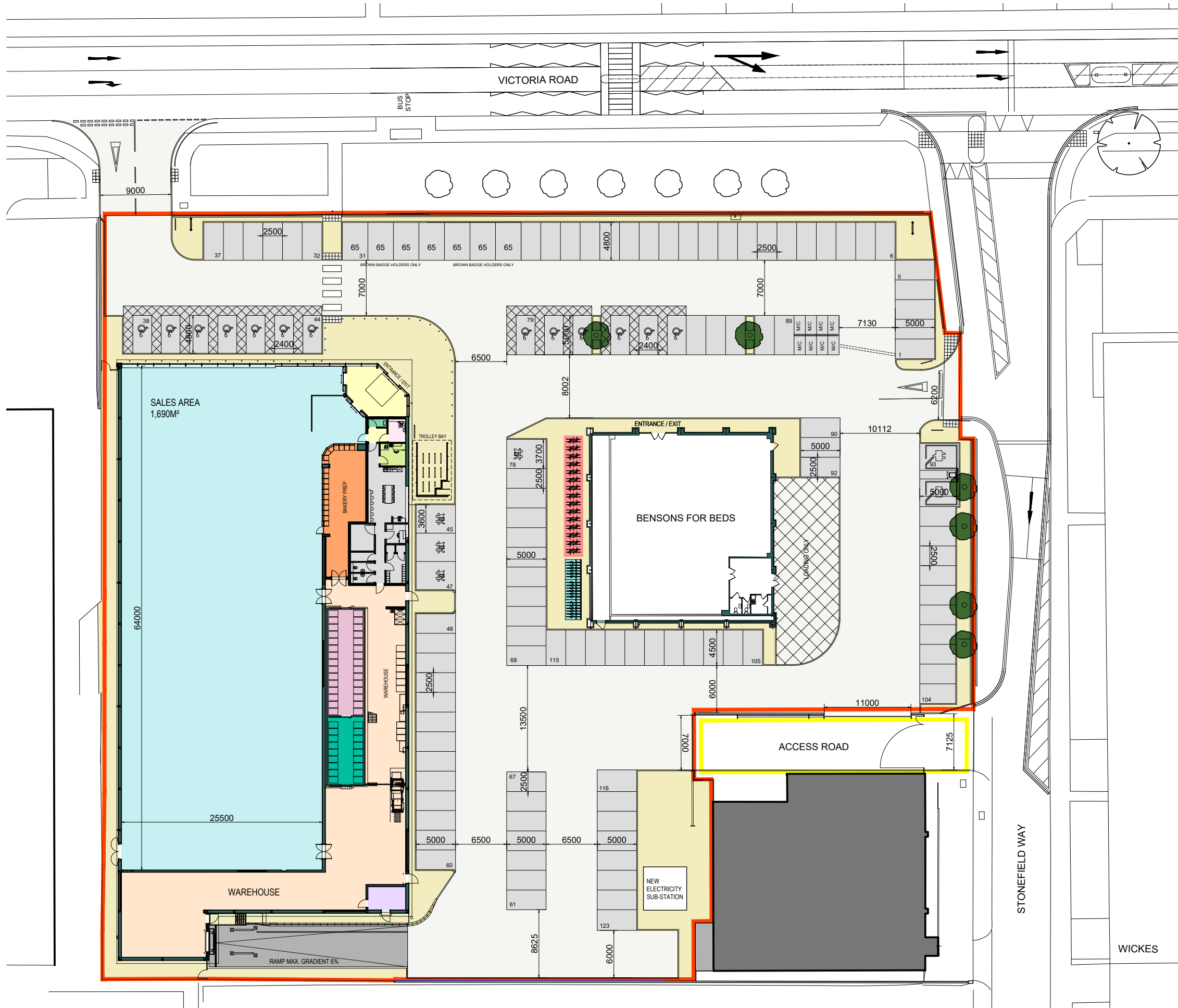
Appendix I – Detailed Maintenance Schedule

APPENDIX A

RSK GROUP SERVICE CONSTRAINTS

1. This report and the drainage design carried out in connection with the report (together the "Services") were compiled and carried out by RSK LDE Ltd (RSK) for LIDL GB (the "client") in accordance with the terms of a contract between RSK and the "client" dated 23rd October 2023. The Services were performed by RSK with the skill and care ordinarily exercised by a reasonable civil engineer at the time the Services were performed. Further, and in particular, the Services were performed by RSK considering the limits of the scope of works required by the client, the time scale involved and the resources, including financial and manpower resources, agreed between RSK and the client.
2. Other than that expressly contained in paragraph 1 above, RSK provides no other representation or warranty whether express or implied, in relation to the Services.
3. Unless otherwise agreed in writing, the Services were performed by RSK exclusively for the purposes of the client. RSK is not aware of any interest of or reliance by any party other than the client in or on the Services. Unless expressly provided in writing, RSK does not authorise, consent or condone any party other than the client relying upon the Services. Should this report or any part of this report, or otherwise details of the Services or any part of the Services be made known to any such party, and such party relies thereon that party does so wholly at its own and sole risk and RSK disclaims any liability to such parties. Any such party would be well advised to seek independent advice from a competent environmental consultant and/or lawyer.
4. It is RSK's understanding that this report is to be used for the purpose described in the introduction to the report. That purpose was a significant factor in determining the scope and level of the Services. Should the purpose for which the report is used, or the proposed use of the site change, this report may no longer be valid and any further use of or reliance upon the report in those circumstances by the client without RSK's review and advice shall be at the client's sole and own risk. Should RSK be requested to review the report after the date of this report, RSK shall be entitled to additional payment at the then existing rates or such other terms as agreed between RSK and the client.
5. The passage of time may result in changes in site conditions, regulatory or other legal provisions, technology or economic conditions which could render the report inaccurate or unreliable. The information and conclusions contained in this report should not be relied upon in the future without the written advice of RSK. In the absence of such written advice of RSK, reliance on the report in the future shall be at the client's own and sole risk. Should RSK be requested to review the report in the future, RSK shall be entitled to additional payment at the then existing rate or such other terms as may be agreed between RSK and the client.
6. The observations and conclusions described in this report are based solely upon the Services, which were provided pursuant to the agreement between the client and RSK. RSK has not performed any observations, investigations, studies or testing not specifically set out or required by the contract between the client and RSK. RSK is not liable for the existence of any condition, the discovery of which would require performance of services not otherwise contained in the Services. For the avoidance of doubt, unless otherwise expressly referred to in the introduction to this report, RSK did not seek to evaluate the presence on or off the site of asbestos, electromagnetic fields, lead paint, heavy metals, radon gas or other radioactive or hazardous materials.
7. The Services are based upon RSK's observations of existing physical conditions at the site gained from a walk-over survey of the site together with RSK's interpretation of information including documentation, obtained from third parties and from the client on the history and usage of the site. The Services are also based on information and/or analysis provided by independent testing and information services or laboratories upon which RSK was reasonably entitled to rely. The Services clearly are limited by the accuracy of the information, including documentation, reviewed by RSK and the observations possible at the time of the walk-over survey. Further RSK was not authorised and did not attempt to independently verify the accuracy or completeness of information, documentation or materials received from the client or third parties, including laboratories and information services, during the performance of the Services. RSK is not liable for any inaccurate information or conclusions, the discovery of which inaccuracies required the doing of any act including the gathering of any information which was not reasonably available to RSK and including the doing of any independent investigation of the information provided to RSK save as otherwise provided in the terms of the contract between the client and RSK.
8. The phase II or intrusive environmental site investigation aspects of the Services is a limited sampling of the site at pre-determined borehole and soil vapour locations based on the operational configuration of the site. The conclusions given in this report are based on information gathered at the specific test locations and can only be extrapolated to an undefined limited area around those locations. The extent of the limited area depends on the soil and groundwater conditions, together with the position of any current structures and underground facilities and natural and other activities on site. In addition chemical analysis was carried out for a limited number of parameters [as stipulated in the contract between the client and RSK] [based on an understanding of the available operational and historical information,] and it should not be inferred that other chemical species are not present.
9. Any site drawing(s) provided in this report is (are) not meant to be an accurate base plan, but is (are) used to present the general relative locations of features on, and surrounding, the site. Features (boreholes, trial pits etc) annotated on site plans are not drawn to scale but are centred over the appropriate location. Such features should not be used for setting out and should be considered indicative only.

APPENDIX B **EXISTING SITE PLAN / TOPOGRAPHICAL SURVEY**



NO DIMENSIONS TO BE SCALED FROM THIS DRAWING

Rev Date Reference Drawn / Chk'd

P01 2020/01/28 P01 FIRST ISSUE AA AA

SCHEDULE OF ACCOMMODATION - KEY

| | |
|--|----------------------|
| Site Area | 0.94 ha (2.32 acres) |
| Right of Way | sq m |
| GIA | 2,552 sq m |
| GEA | 2,635 sq m |
| Sales area | 1,690 sq m |
| WAREHOUSE | |
| Warehouse | 482 sq m |
| Bakery Warehouse | 63 sq m |
| Additional Chillers | 40 sq m |
| Total Warehouse | 585 sq m |
| ANCILLARY AREA | |
| Bakery Prep | 66 sq m |
| Cash Office | 9 sq m |
| Cleaning Room | 11 sq m |
| Welfare area, wcs, etc | 92 sq m |
| Customer WC | 6 sq m |
| Utility | 14 sq m |
| Circulation | 30 sq m |
| Internal partitions | 49 sq m |
| Total Ancillary | 277 sq m |
| PARKING | |
| Standard | 97 |
| Disabled | 13 |
| Brown Badge | 7 |
| Parent & child | 4 |
| EVCP Active Terra S3 C/JG charging station | 2 |
| TOTAL | 123 spaces |
| Motor cycles | M/C 8 |
| Short stay cycles | 30 |
| Long stay cycles (covered) | 18 |

Client

Lidl Great Britain Ltd

Project

Lidl
Victoria Road, South Ruislip

Title

Site Plan as Existing

Drawing Ref.

4908-0100

Revision

P01

Scale - unless otherwise stated

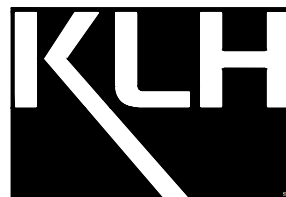
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Status

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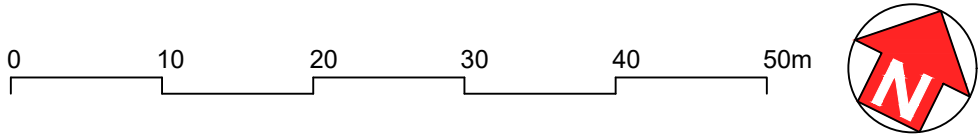
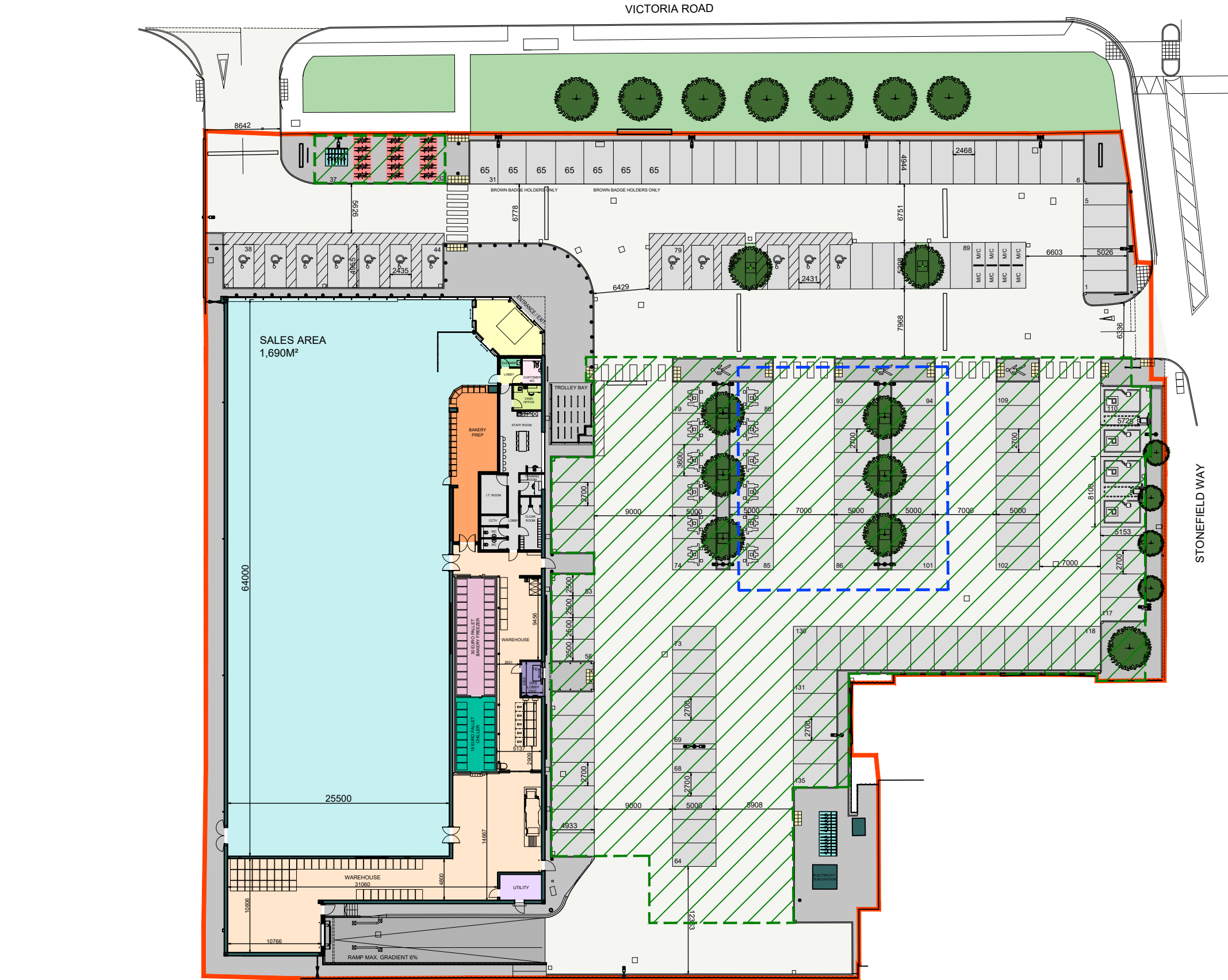
Issued For

Preliminary



The Old Steelyard, Poplar Lane
Sroughton, Ipswich, IP8 3HL
t.01473 689532
klh@klharchitects.com
www.klharchitects.com

APPENDIX C PROPOSED SITE PLAN



PARKING - KEY

| | | | | | |
|----------------------|----|-----|--------------------------|-----|------------|
| Standard | | 99 | Motor Cycles | M/C | 8 |
| Disabled | | 13 | Short Stay Cycle Parking | | 30 |
| Brown Badge | 65 | 7 | Long Stay Cycle Parking | | 18 |
| Parent & Child | | 12 | | | |
| EVCP Active Terra S3 | | 4 | EXTERNAL WORKS | | |
| CJG charging station | | | Re-surfacing Works | | 3,217 sq m |
| Total Parking | | 135 | To be Demolished | | |

NO DIMENSIONS TO BE SCALED FROM THIS DRAWING

| Rev | Date | Reference | Drawn / Chk'd |
|-----|------------|---------------------------|---------------|
| P11 | 2023/01/06 | PARKING SCHEDULE REVISED. | AA AA |

SCHEDULE OF ACCOMMODATION - KEY

| | | |
|----------------|------------------------|----------------------|
| | Site Area | 0.94 ha (2.32 acres) |
| | Right of Way | |
| | GIA | 2,552 sq m |
| | GEA | 2,636 sq m |
| | Sales Area | 1,690 sq m |
| WAREHOUSE | | |
| | Warehouse | 458 sq m |
| | Bakery Warehouse | 63 sq m |
| | Additional Chillers | 40 sq m |
| | DRS Lobby | 10 sq m |
| | Total Warehouse | 571 sq m |
| ANCILLARY AREA | | |
| | Bakery Prep | 78 sq m |
| | Cash Office | 9 sq m |
| | Cleaner's Cupboard | 2 sq m |
| | Welfare area, wcs, etc | 100 sq m |
| | Customer WC | 6 sq m |
| | Utility | 14 sq m |
| | Circulation | 43 sq m |
| | Internal Partitions | 39 sq m |
| | Total Ancillary | 291 sq m |

Client

Lidl Great Britain Ltd

Project

Lidl
Victoria Road, South Ruislip

Title

Site Plan as Proposed - Option C

Drawing Ref.

4908-0104

Revision

P11

Scale - unless otherwise stated

1:500 @ A3

Status

S0

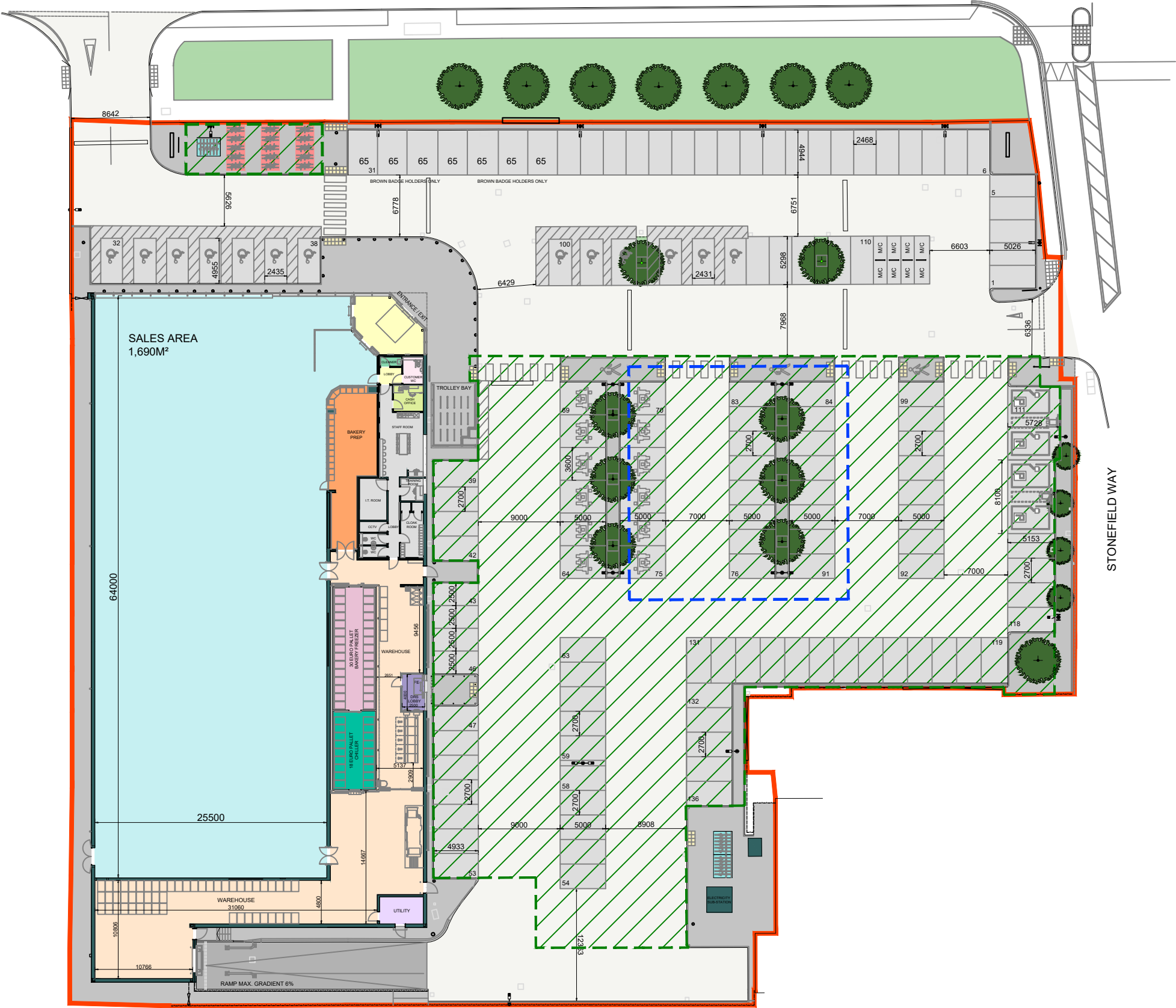
Issued For

Preliminary

KLH architects

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Sproughton, Ipswich, IP8 3HL
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www.klharchitects.com

VICTORIA ROAD



STONEFIELD WAY

Client

Lidl Great Britain Ltd

Project

Lidl
Victoria Road, South Ruislip

Title

Soft Landscaping Plan

Drawing Ref. Revision

4908-0109 P03

| Scale - unless otherwise stated | Status | Issued For |
|---------------------------------|--------|-------------|
| 1:500 @ A3 | S0 | Preliminary |

0 10 20 30 40 50m



APPENDIX D THAMES WATER SEWER RECORDS

RSK Land And Development Engineering Ltd
18Frogmore Road Frogmore Road In
HEMEL HEMPSTEAD
HP3 9RT

Search address supplied Lidl South Ruislip
Victoria Road
South Ruislip
HA4 0QQ

Your reference 890410

Our reference ALS/ALS Standard/2020_4193536

Search date 2 June 2020

Knowledge of features below the surface is essential for every development

The benefits of this knowledge not only include ensuring due diligence and avoiding risk, but also being able to ascertain the feasibility of any development.

Did you know that Thames Water Property Searches can also provide a variety of utility searches including a more comprehensive view of utility providers' assets (across up to 35-45 different providers), as well as more focused searches relating to specific major utility companies such as National Grid (gas and electric).

Contact us to find out more.



Thames Water Utilities Ltd
Property Searches, PO Box 3189, Slough SL1 4WW
DX 151280 Slough 13



searches@thameswater.co.uk
www.thameswater-propertysearches.co.uk



0845 070 9148

Search address supplied: Lidl South Ruislip, Victoria Road, South Ruislip, HA4 0QQ

Dear Sir / Madam

An Asset Location Search is recommended when undertaking a site development. It is essential to obtain information on the size and location of clean water and sewerage assets to safeguard against expensive damage and allow cost-effective service design.

The following records were searched in compiling this report: - the map of public sewers & the map of waterworks. Thames Water Utilities Ltd (TWUL) holds all of these.

This search provides maps showing the position, size of Thames Water assets close to the proposed development and also manhole cover and invert levels, where available.

Please note that none of the charges made for this report relate to the provision of Ordnance Survey mapping information. The replies contained in this letter are given following inspection of the public service records available to this company. No responsibility can be accepted for any error or omission in the replies.

You should be aware that the information contained on these plans is current only on the day that the plans are issued. The plans should only be used for the duration of the work that is being carried out at the present time. Under no circumstances should this data be copied or transmitted to parties other than those for whom the current work is being carried out.

Thames Water do update these service plans on a regular basis and failure to observe the above conditions could lead to damage arising to new or diverted services at a later date.

Contact Us

If you have any further queries regarding this enquiry please feel free to contact a member of the team on 0845 070 9148, or use the address below:

Thames Water Utilities Ltd
Property Searches
PO Box 3189
Slough
SL1 4WW

Email: searches@thameswater.co.uk

Web: www.thameswater-propertysearches.co.uk

Waste Water Services

Please provide a copy extract from the public sewer map.

Enclosed is a map showing the approximate lines of our sewers. Our plans do not show sewer connections from individual properties or any sewers not owned by Thames Water unless specifically annotated otherwise. Records such as "private" pipework are in some cases available from the Building Control Department of the relevant Local Authority.

Where the Local Authority does not hold such plans it might be advisable to consult the property deeds for the site or contact neighbouring landowners.

This report relates only to sewerage apparatus of Thames Water Utilities Ltd, it does not disclose details of cables and or communications equipment that may be running through or around such apparatus.

The sewer level information contained in this response represents all of the level data available in our existing records. Should you require any further Information, please refer to the relevant section within the 'Further Contacts' page found later in this document.

For your guidance:

- The Company is not generally responsible for rivers, watercourses, ponds, culverts or highway drains. If any of these are shown on the copy extract they are shown for information only.
- Any private sewers or lateral drains which are indicated on the extract of the public sewer map as being subject to an agreement under Section 104 of the Water Industry Act 1991 are not an 'as constructed' record. It is recommended these details be checked with the developer.

Clean Water Services

Please provide a copy extract from the public water main map.

With regard to the fresh water supply, this site falls within the boundary of another water company. For more information, please redirect your enquiry to the following address:

Affinity Water Ltd
Tamblin Way
Hatfield
AL10 9EZ
Tel: 0345 3572401



For your guidance:

- Assets other than vested water mains may be shown on the plan, for information only.
- If an extract of the public water main record is enclosed, this will show known public water mains in the vicinity of the property. It should be possible to estimate the likely length and route of any private water supply pipe connecting the property to the public water network.

Payment for this Search

A charge will be added to your suppliers account.

Further contacts:

Waste Water queries

Should you require verification of the invert levels of public sewers, by site measurement, you will need to approach the relevant Thames Water Area Network Office for permission to lift the appropriate covers. This permission will usually involve you completing a TWOSA form. For further information please contact our Customer Centre on Tel: 0845 920 0800. Alternatively, a survey can be arranged, for a fee, through our Customer Centre on the above number.

If you have any questions regarding sewer connections, budget estimates, diversions, building over issues or any other questions regarding operational issues please direct them to our service desk. Which can be contacted by writing to:

Developer Services (Waste Water)
Thames Water
Clearwater Court
Vastern Road
Reading
RG1 8DB

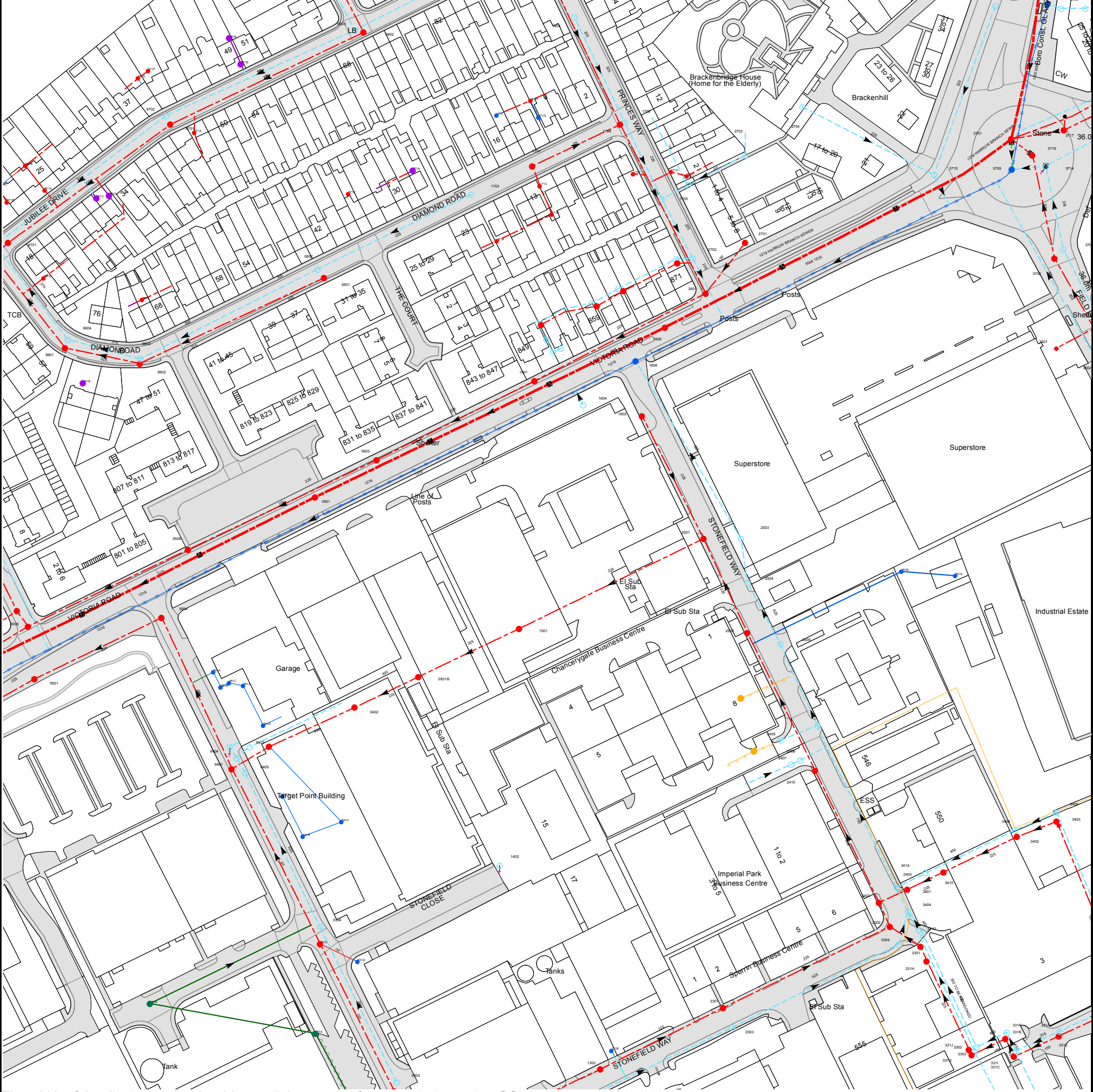
Tel: 0800 009 3921
Email: developer.services@thameswater.co.uk

Clean Water queries

Should you require any advice concerning clean water operational issues or clean water connections, please contact:

Developer Services (Clean Water)
Thames Water
Clearwater Court
Vastern Road
Reading
RG1 8DB

Tel: 0800 009 3921
Email: developer.services@thameswater.co.uk



The width of the displayed area is 500 m and the centre of the map is located at OS coordinates 512141,185571
The position of the apparatus shown on this plan is given without obligation and warranty, and the accuracy cannot be guaranteed. Service pipes are not shown but their presence should be anticipated. No liability of any kind whatsoever is accepted by Thames Water for any error or omission. The actual position of mains and services must be verified and established on site before any works are undertaken.

Based on the Ordnance Survey Map with the Sanction of the controller of H.M. Stationery Office, License no. 100019345 Crown Copyright Reserved.

NB. Levels quoted in metres Ordnance Newlyn Datum. The value -9999.00 indicates that no survey information is available

| Manhole Reference | Manhole Cover Level | Manhole Invert Level |
|-------------------|---------------------|----------------------|
| 971D | n/a | n/a |
| 971E | n/a | n/a |
| 961C | n/a | n/a |
| 971F | n/a | n/a |
| 9702 | 34.81 | 33.44 |
| 9701 | 34.85 | 32.76 |
| 971A | n/a | n/a |
| 981B | n/a | n/a |
| 071B | n/a | n/a |
| 0602 | 34.88 | 33.59 |
| 0601 | 36 | 34.2 |
| 071C | n/a | n/a |
| 0803 | 34.92 | 33.61 |
| 0802 | 34.91 | 33 |
| 071A | n/a | n/a |
| 1703 | 34.94 | 33.92 |
| 171F | n/a | n/a |
| 171A | n/a | n/a |
| 171H | n/a | n/a |
| 1701 | 35.05 | 33.32 |
| 2702 | 35.11 | 33.72 |
| 2601 | 35.07 | 32.81 |
| 2703 | 35.56 | 34.71 |
| 2701 | 34.99 | 33.01 |
| 2704 | 34.98 | 34.07 |
| 3715 | 35.69 | 33.77 |
| 3807 | 35.98 | 34.26 |
| 3701 | 36.79 | 31.38 |
| 3705 | 36.77 | 33.75 |
| 3716 | 37.04 | 33.22 |
| 3714 | 37.82 | 35.47 |
| 3803 | 36.08 | 34.01 |
| 3702 | 36.02 | 33.48 |
| 3601 | 35.94 | 34.18 |
| 3802 | 36.02 | 34.83 |
| 3701B | 36.01 | 34.79 |
| 3717 | 36.5 | 32.18 |
| 3801B | 36 | 34.95 |
| 1612 | n/a | n/a |
| 171G | n/a | n/a |
| 171C | n/a | n/a |
| 1615 | n/a | n/a |
| 1611 | n/a | n/a |
| 171B | n/a | n/a |
| 1610 | n/a | n/a |
| 1609 | n/a | n/a |
| 1804 | 35.07 | 34.17 |
| 1613 | n/a | n/a |
| 1616 | n/a | n/a |
| 1614 | n/a | n/a |
| 1702 | 34.88 | 33.07 |
| 1617 | n/a | n/a |
| 1704 | 34.89 | 33.94 |
| 171E | n/a | n/a |
| 1705 | 34.97 | 33.82 |
| 1606 | 35.07 | 30.97 |
| 171D | n/a | n/a |
| 1706 | n/a | n/a |
| 1618 | n/a | n/a |
| 271B | n/a | n/a |
| 271C | n/a | n/a |
| 271A | n/a | n/a |
| 9404 | 34.37 | 32.81 |
| 0406 | n/a | n/a |
| 0407 | n/a | n/a |
| 041D | n/a | n/a |
| 0402 | 34.72 | 32.27 |
| 951B | n/a | n/a |
| 051A | n/a | n/a |
| 951C | n/a | n/a |
| 0501B | 34.68 | 32.31 |
| 951A | n/a | n/a |
| 9504 | 34.68 | 31.88 |
| 9506 | 34.63 | 31.76 |
| 0501 | 34.52 | 30.46 |
| 0603 | 34.62 | 31.99 |
| 9602 | 34.79 | 32.78 |
| 9603 | 34.74 | 33.39 |
| 3401 | 36 | 33.55 |
| 2403 | 36.01 | 34.01 |
| 341A | 36.32 | n/a |
| 3410 | 36.14 | 33.62 |
| 1402 | n/a | n/a |
| 3402 | 36.04 | 34.04 |
| 3406 | 36.04 | 34.53 |
| 3403 | 36.72 | 34.29 |
| 3405 | 36.72 | 34.6 |
| 2410 | n/a | n/a |
| 2407 | 35.55 | 34.22 |
| 2408 | 35.74 | 34.16 |
| 2405 | 36.43 | 34.96 |

| Manhole Reference | Manhole Cover Level | Manhole Invert Level |
|--|---------------------|----------------------|
| 2411 | n/a | n/a |
| 2505 | 35.44 | 34.16 |
| 2502 | 35.47 | 33.11 |
| 1501 | 34.98 | 32.43 |
| 2504 | 35.29 | 33.66 |
| 351A | n/a | n/a |
| 351B | n/a | n/a |
| 2501 | 35.2 | 32.66 |
| 2503 | 35.23 | 33.69 |
| 1602 | 35.02 | 33.16 |
| 1604 | 34.98 | 33.72 |
| 1601 | 34.72 | 32.1 |
| 1608 | 35.01 | n/a |
| 0302 | 36.86 | 34.61 |
| 0303 | 36.81 | 35.04 |
| 031A | n/a | n/a |
| 931A | n/a | n/a |
| 031D | n/a | n/a |
| 031C | n/a | n/a |
| 0304 | 35.58 | 33.77 |
| 041B | n/a | n/a |
| 041C | n/a | n/a |
| 041A | n/a | n/a |
| 0405 | 34.31 | 32.85 |
| 9401 | 34.39 | 31.89 |
| 9403 | 34.35 | 32.78 |
| 1302 | 37.2 | 34.6 |
| 131B | n/a | n/a |
| 2301 | 36.68 | 34.25 |
| 2303 | 36.65 | 34.28 |
| 2402 | 36.16 | 33.46 |
| 2302 | 36.11 | 33.54 |
| 3304 | 36.41 | 34.03 |
| 3404 | 36.17 | 33.93 |
| 3301 | 36.61 | 33.76 |
| 331D | 36.31 | 33.96 |
| 331H | 36.68 | 33.9 |
| 3302 | n/a | n/a |
| 331G | 36.82 | 34.55 |
| 3303 | 36.74 | 34.94 |
| 331J | 36.73 | 34.16 |
| 331B | 36.91 | 35.03 |
| 331F | 36.92 | 34.21 |
| 331C | 37 | 35.11 |
| 331I | 36.96 | 34.33 |
| 331E | 37.26 | 35.33 |
| 331A | 37.22 | 34.51 |
| 871B | n/a | n/a |
| 8701 | 34.79 | 32.47 |
| 8502 | 34.48 | 31.41 |
| 971B | n/a | n/a |
| 9505 | 34.47 | 31.42 |
| 9501 | 34.63 | 31.72 |
| 961D | n/a | n/a |
| 871A | n/a | n/a |
| 9601 | 35.01 | 32.61 |
| 9604 | 35.02 | 33.39 |
| 961B | n/a | n/a |
| 971C | n/a | n/a |
| The position of the apparatus shown on this plan is given without obligation and warranty, and the accuracy cannot be guaranteed. Service pipes are not shown but their presence should be anticipated. No liability of any kind whatsoever is accepted by Thames Water for any error or omission. The actual position of mains and services must be verified and established on site before any works are undertaken. | | |



ALS Sewer Map Key

Public Sewer Types (Operated & Maintained by Thames Water)

| | | | |
|--|---|--|-----------------------------------|
| | Foul: A sewer designed to convey waste water from domestic and industrial sources to a treatment works. | | Trunk Foul |
| | Surface Water: A sewer designed to convey surface water (e.g. rain water from roofs, yards and car parks) to rivers or watercourses. | | Trunk Surface Water |
| | Combined: A sewer designed to convey both waste water and surface water from domestic and industrial sources to a treatment works. | | Trunk Combined |
| | Storm Relief | | Bio-solids (Sludge) |
| | Vent Pipe | | Proposed Thames Water Foul Sewer |
| | Proposed Thames Surface Water Sewer | | Foul Rising Main |
| | Gallery | | Combined Rising Main |
| | Surface Water Rising Main | | Proposed Thames Water Rising Main |
| | Sludge Rising Main | | Vacuum |

Notes:

- 1) All levels associated with the plans are to Ordnance Datum Newlyn.
- 2) All measurements on the plans are metric.
- 3) Arrows (on gravity fed sewers) or flecks (on rising mains) indicate direction of flow.
- 4) Most private pipes are not shown on our plans, as in the past, this information has not been recorded.
- 5) 'na' or '0' on a manhole level indicates that data is unavailable.

Sewer Fittings

A feature in a sewer that does not affect the flow in the pipe. Example: a vent is a fitting as the function of a vent is to release excess gas.

| | |
|--|-------------|
| | Air Valve |
| | Dam Chase |
| | Fitting |
| | Meter |
| | Vent Column |

Operational Controls

A feature in a sewer that changes or diverts the flow in the sewer. Example: A hydrobrake limits the flow passing downstream.

| | |
|--|---------------|
| | Control Valve |
| | Drop Pipe |
| | Ancillary |
| | Weir |

End Items

End symbols appear at the start or end of a sewer pipe. Examples: an Undefined End at the start of a sewer indicates that Thames Water has no knowledge of the position of the sewer upstream of that symbol, Outfall on a surface water sewer indicates that the pipe discharges into a stream or river.

| | |
|--|---------------|
| | Outfall |
| | Undefined End |
| | Inlet |

Other Symbols

Symbols used on maps which do not fall under other general categories

| | |
|--|---|
| | Public/Private Pumping Station |
| | Change of characteristic indicator (C.O.C.I.) |
| | Invert Level |
| | Summit |

Areas

Lines denoting areas of underground surveys, etc.

| | |
|--|------------------|
| | Agreement |
| | Operational Site |
| | Chamber |
| | Tunnel |
| | Conduit Bridge |

Other Sewer Types (Not Operated or Maintained by Thames Water)

| | | | |
|--|-----------------------|--|---------------------|
| | Foul Sewer | | Surface Water Sewer |
| | Combined Sewer | | Gully |
| | Culverted Watercourse | | Proposed |
| | | | Abandoned Sewer |

- 6) The text appearing alongside a sewer line indicates the internal diameter of the pipe in millimetres. Text next to a manhole indicates the manhole reference number and should not be taken as a measurement. If you are unsure about any text or symbology present on the plan, please contact a member of Property Insight on 0845 070 9148.

Terms and Conditions

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1. All goods remain in the property of Thames Water Utilities Ltd until full payment is received.
2. Provision of service will be in accordance with all legal requirements and published TWUL policies.
3. All invoices are strictly due for payment 14 days from due date of the invoice. Any other terms must be accepted/agreed in writing prior to provision of goods or service, or will be held to be invalid.
4. Thames Water does not accept post-dated cheques-any cheques received will be processed for payment on date of receipt.
5. In case of dispute TWUL's terms and conditions shall apply.
6. Penalty interest may be invoked by TWUL in the event of unjustifiable payment delay. Interest charges will be in line with UK Statute Law 'The Late Payment of Commercial Debts (Interest) Act 1998'.
7. Interest will be charged in line with current Court Interest Charges, if legal action is taken.
8. A charge may be made at the discretion of the company for increased administration costs.

A copy of Thames Water's standard terms and conditions are available from the Commercial Billing Team (cashoperations@thameswater.co.uk).

We publish several Codes of Practice including a guaranteed standards scheme. You can obtain copies of these leaflets by calling us on 0800 316 9800

If you are unhappy with our service you can speak to your original goods or customer service provider. If you are not satisfied with the response, your complaint will be reviewed by the Customer Services Director. You can write to her at: Thames Water Utilities Ltd. PO Box 492, Swindon, SN38 8TU.

If the Goods or Services covered by this invoice falls under the regulation of the 1991 Water Industry Act, and you remain dissatisfied you can refer your complaint to Consumer Council for Water on 0121 345 1000 or write to them at Consumer Council for Water, 1st Floor, Victoria Square House, Victoria Square, Birmingham, B2 4AJ.

Ways to pay your bill

| Credit Card | BACS Payment | Telephone Banking | Cheque |
|---|--|--|--|
| Call 0845 070 9148 quoting your invoice number starting CBA or ADS / OSS | Account number 90478703 Sort code 60-00-01 A remittance advice must be sent to: Thames Water Utilities Ltd., PO Box 3189, Slough SL1 4WW. or email ps.billing@thameswater.co.uk | By calling your bank and quoting: Account number 90478703 Sort code 60-00-01 and your invoice number | Made payable to ' Thames Water Utilities Ltd ' Write your Thames Water account number on the back. Send to: Thames Water Utilities Ltd., PO Box 3189, Slough SL1 4WW or by DX to 151280 Slough 13 |

Thames Water Utilities Ltd Registered in England & Wales No. 2366661 Registered Office Clearwater Court, Vastern Rd, Reading, Berks, RG1 8DB.

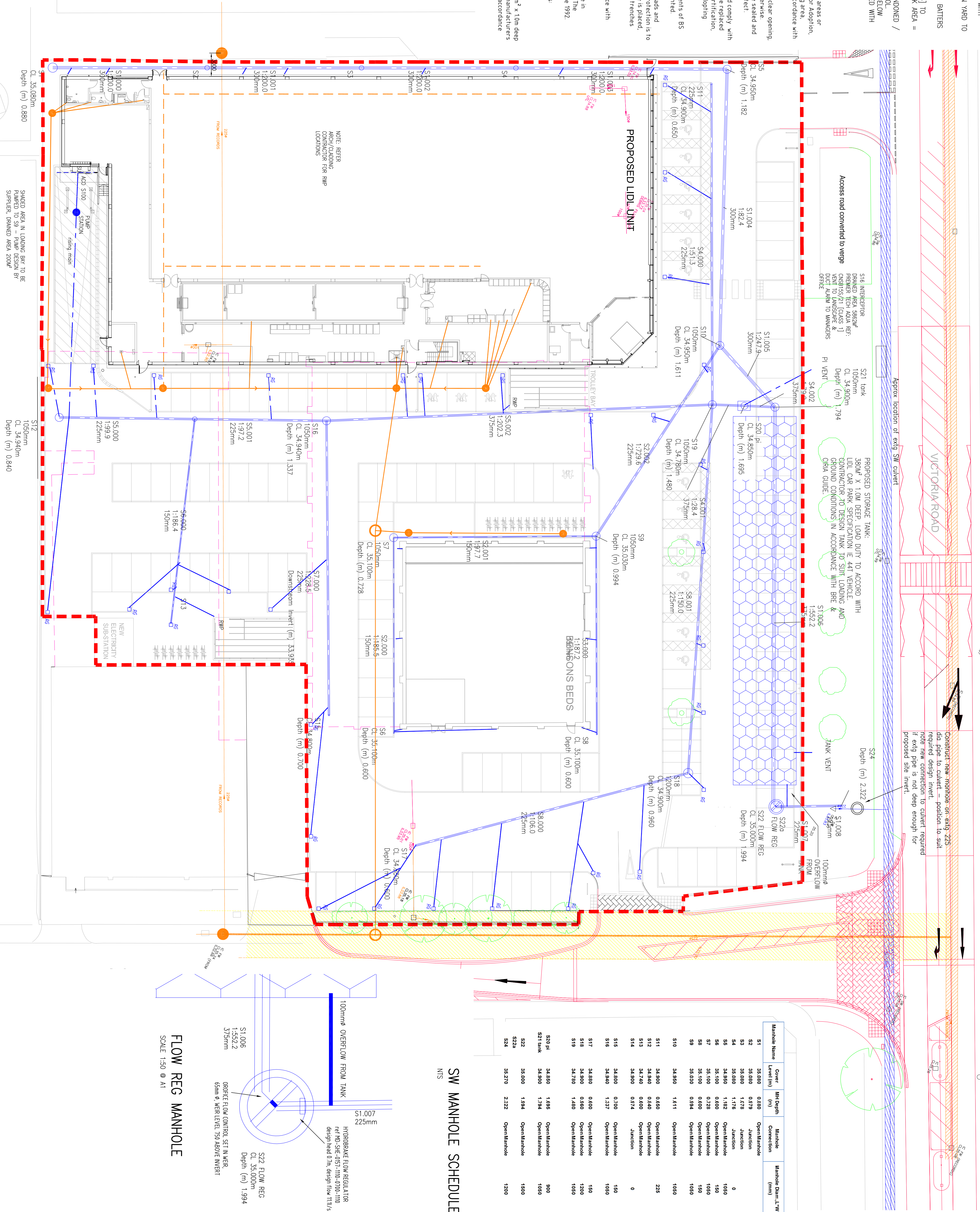
APPENDIX E **EXISTING DRAINAGE PLAN**

NOTE:

- ALL GULLY CONNECTIONS TO BE 150 DIA AT 180
- GULLIES TO BE IN ACCORDANCE WITH LID STANDARD FOR POSITION IN PARKING BAYS & MANUFACTURER WORKS TO PUBLIC SEWERS IN ACCORDANCE WITH
- S106 AGREEMENTS & SFA
- PUMPING STATION TO ACO S100 CHANNEL IN YARD TO SUPPLIES DESIGN & DETAILS
- TANK TO BE INSTALLED IN ACCORDANCE WITH MANUFACTURERS DETAILS UTILISING SUITABLE BATTERS OR EARTHWORKS SUPPORT.
- CLASS 1 PETROL INTERCEPTOR (MANHOLE S3) TO MANUFACTURERS DETAILS, DRAINED CAR PARK AREA = 5862m²
- ALL EXISTING PRIVATE DRAINAGE TO BE ABANDONED / REMOVED TO APPROVAL OF BUILDING CONTROL.
- MANHOLES TO BE REMOVED TO 1000 BELOW PROPOSED GROUND LEVEL & BACKFILLED WITH LEAN MIX
- PIPES TO BE GROUTED

NOTES

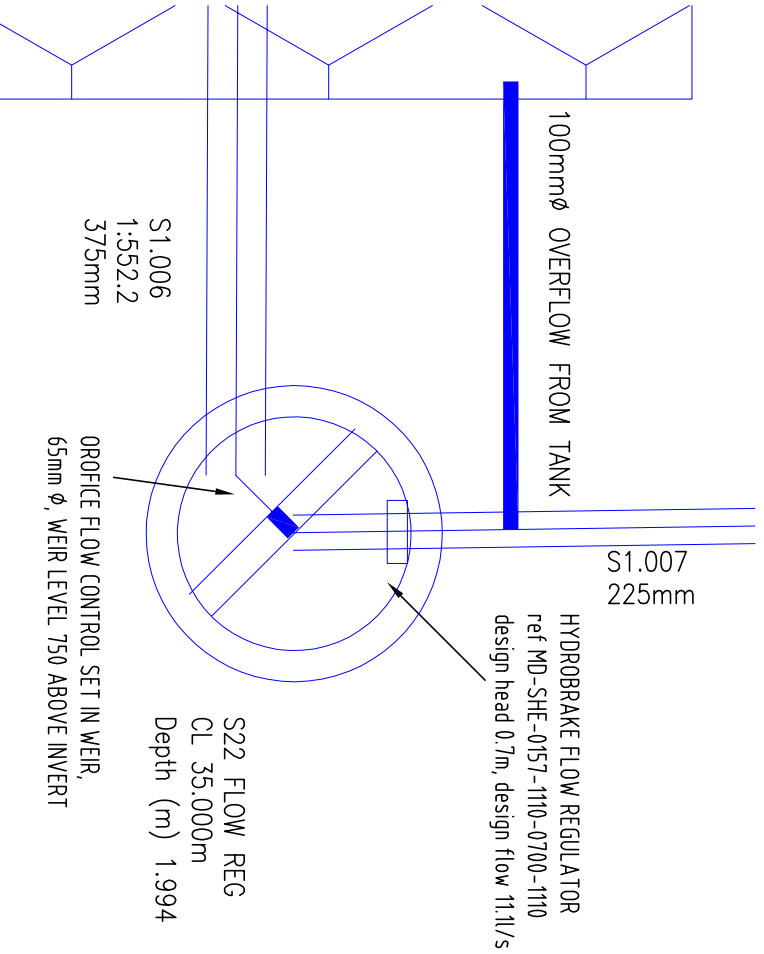
1. All works and materials in heavily trafficked areas or highways to be in accordance with Sewers for Adoption, 5th edition, 2001. All drainage within building area, non-trafficked hardstandings etc to be in accordance with building regulations.
2. Manhole covers and frames to be 600 x 600 clear opening, in accordance with EN 124, unless noted otherwise. Manhole covers within buildings to be double sealed and recessed to suit finish as required by Architect
3. Vitrified clay pipes to be flexibly jointed and comply with the requirements of EN 295. Clayware may be replaced with suitable PE products with Agrément Certification, subject to approval of Building Control / Adopting Authority.
4. Concrete pipes to comply with the requirements of BS 5911, class M strength and to be flexibly jointed.
5. Where cover to pipes is less than 1200 in roads and hardstandings or 900 elsewhere concrete protection is to be provided as detail. NB Until final surface is placed, heavy traffic is not to be allowed over pipe trenches without special provisions.
6. Pipes and fittings are to be laid in accordance with Manufacturers recommendations.
7. Permanent trench reinstatements are to be in accordance with the HAUC Specification for The Reinstatement Of Openings in Highways, June 1992.
8. Hydrobrake flow control required as follows:
ref MD-SHE-053-T110-1000-T110
design head 10m, design flow 11l/s
9. Surface water storage tank comprising 34.0m³ x 10m deep cellular tank, min cover in accordance with manufacturers requirements to suit 4.4t vehicle loading in accordance with Lidl Spec.



SW MANHOLE SCHEDULE

| Manhole Name | Cover Level (m) | MH Depth (m) | Manhole Connection | Manhole Diam. L-W (mm) | Pipe Out Invert Level (m) | Pipe Out Diameter (mm) | Pipes In Invert Level (m) | Pipes In Diameter (mm) | Pipes In Backdrop (mm) |
|--------------|-----------------|--------------|--------------------|------------------------|---------------------------|------------------------|---------------------------|------------------------|------------------------|
| S1 | 35.880 | 0.880 | Open/Manhole | 900 | 31.000 | 34.000 | 300 | 51.000 | 34.100 |
| S2 | 35.880 | 0.879 | Junction | 150 | 31.001 | 34.001 | 300 | 51.001 | 34.101 |
| S3 | 35.880 | 1.078 | Junction | 150 | 31.002 | 34.002 | 300 | 51.002 | 34.102 |
| S4 | 35.880 | 1.178 | Junction | 0 | 31.003 | 33.904 | 300 | 51.003 | 33.904 |
| S5 | 34.950 | 1.182 | Open/Manhole | 1850 | 32.001 | 33.786 | 300 | 51.003 | 33.768 |
| S6 | 35.100 | 0.600 | Open/Manhole | 1850 | 32.001 | 34.972 | 150 | 52.000 | 34.372 |
| S7 | 35.100 | 0.728 | Open/Manhole | 1850 | 32.001 | 34.800 | 150 | 52.001 | 34.111 |
| S8 | 35.030 | 0.994 | Open/Manhole | 1850 | 32.002 | 34.038 | 225 | 52.001 | 34.111 |
| S9 | 34.950 | 1.411 | Open/Manhole | 1850 | 31.005 | 33.339 | 300 | 51.004 | 33.339 |
| S10 | 34.950 | 0.650 | Open/Manhole | 225 | 34.000 | 34.450 | 225 | 52.002 | 33.996 |
| S11 | 34.900 | 0.650 | Open/Manhole | 150 | 35.000 | 34.100 | 225 | 52.001 | 34.079 |
| S12 | 34.940 | 0.640 | Open/Manhole | 150 | 35.000 | 34.100 | 225 | 52.000 | 33.926 |
| S13 | 34.740 | 0.600 | Open/Manhole | 0 | 35.001 | 33.926 | 225 | 52.000 | 33.926 |
| S14 | 34.900 | 0.714 | Junction | 150 | 37.000 | 34.100 | 225 | 52.001 | 33.763 |
| S15 | 34.900 | 0.700 | Open/Manhole | 150 | 35.002 | 33.893 | 375 | 52.001 | 33.763 |
| S16 | 34.900 | 1.337 | Open/Manhole | 150 | 35.002 | 33.893 | 375 | 52.001 | 33.763 |
| S17 | 34.980 | 0.800 | Open/Manhole | 150 | 38.000 | 34.280 | 225 | 52.000 | 33.940 |
| S18 | 34.900 | 0.560 | Open/Manhole | 1200 | 38.001 | 33.940 | 225 | 52.000 | 33.940 |
| S19 | 34.780 | 1.480 | Open/Manhole | 1850 | 34.001 | 33.300 | 375 | 52.002 | 33.368 |
| S20 PI | 34.950 | 1.698 | Open/Manhole | 900 | 34.002 | 33.156 | 375 | 52.001 | 33.156 |
| S21 tank | 34.900 | 1.794 | Open/Manhole | 1850 | 34.001 | 33.156 | 375 | 52.001 | 33.156 |
| S22 | 35.000 | 1.194 | Open/Manhole | 1850 | 31.007 | 33.108 | 225 | 51.006 | 33.066 |
| S23 | 35.000 | 1.194 | Open/Manhole | 1850 | 31.007 | 32.990 | 225 | 51.007 | 32.990 |
| S24 | 35.270 | 2.222 | Open/Manhole | 1200 | OUTFALL | 32.948 | 225 | 51.008 | 32.948 |

FLOW REG MANHOLE



CHARTERED
CIVIL AND
STRUCTURAL
ENGINEERS

p : Unit 3a, Mercury Court
Manse Lane
Knaresborough HG5 8LF
t : 0844 4140860

client



drawing title
PROPOSED SW DRAINAGE GA

| scale | date | drawn | checked |
|---------|--------|-------|---------|
| AS PLAN | DEC 15 | RSM | RSM |

| status | drawing number | revision |
|--------|----------------|----------|
| TENDER | 2472-S110 | B |

APPENDIX F **PROPOSED SURFACE WATER DRAINAGE STRATEGY**

APPENDIX G

SURFACE WATER DRAINAGE CALCULATIONS



Design Settings

| | | | |
|-----------------------|-------------------|--------------------------------------|---------------|
| Rainfall Methodology | FSR | Maximum Time of Concentration (mins) | 30.00 |
| Return Period (years) | 1 | Maximum Rainfall (mm/hr) | 999.9 |
| Additional Flow (%) | 0 | Minimum Velocity (m/s) | 1.00 |
| FSR Region | England and Wales | Connection Type | Level Soffits |
| M5-60 (mm) | 20.000 | Minimum Backdrop Height (m) | 5.000 |
| Ratio-R | 0.400 | Preferred Cover Depth (m) | 1.200 |
| CV | 0.750 | Include Intermediate Ground | ✓ |
| Time of Entry (mins) | 5.00 | Enforce best practice design rules | ✓ |

Nodes

| Name | Area (ha) | Cover Level (m) | Diameter (mm) | Easting (m) | Northing (m) | Depth (m) |
|---------------|--------------|-----------------------|------------------|----------------|-----------------|--------------|
| 1 | 0.038 | 35.080 | 1050 | 512120.189 | 185505.722 | 0.880 |
| 2 | 0.038 | 35.040 | 1050 | 512111.579 | 185523.498 | 0.939 |
| 3 | 0.038 | 35.000 | 1050 | 512102.989 | 185541.299 | 0.998 |
| 4 | 0.038 | 35.000 | 1050 | 512094.393 | 185559.075 | 1.097 |
| 5 | 0.063 | 34.900 | 1050 | 512084.248 | 185578.466 | 1.132 |
| 10 | 0.060 | 34.950 | 1050 | 512121.767 | 185597.978 | 1.611 |
| 12 | 0.043 | 34.940 | 1050 | 512163.291 | 185527.400 | 0.940 |
| 16 | 0.080 | 34.940 | 1050 | 512151.004 | 185560.523 | 1.175 |
| New Tank | 0.079 | 35.030 | | 512150.184 | 185599.871 | 1.405 |
| 17 | 0.068 | 35.100 | 1050 | 512186.926 | 185589.506 | 1.000 |
| 18 | 0.050 | 34.900 | 1050 | 512172.581 | 185611.672 | 1.100 |
| 7 | 0.033 | 35.100 | 1050 | 512154.491 | 185568.940 | 0.728 |
| 9 | 0.100 | 34.980 | 1050 | 512131.006 | 185595.471 | 0.990 |
| 19 | 0.100 | 34.780 | 1350 | 512126.219 | 185600.229 | 1.480 |
| 21 | 0.020 | 34.850 | 1350 | 512122.923 | 185604.873 | 1.645 |
| Existing Tank | 0.092 | 34.900 | | 512141.262 | 185617.152 | 1.845 |
| 22a | | 35.000 | 1500 | 512164.888 | 185625.877 | 1.994 |
| Outfall | | 34.925 | 1350 | 512163.997 | 185631.256 | 2.247 |

Links

| Name | US Node | DS Node | Length (m) | ks (mm) / n | US IL (m) | DS IL (m) | Fall (m) | Slope (1:X) | Dia (mm) | T of C (mins) | Rain (mm/hr) |
|-------|----------|---------|------------|-------------|-----------|-----------|----------|-------------|----------|---------------|--------------|
| 1.000 | 1 | 2 | 19.751 | 0.600 | 34.200 | 34.101 | 0.099 | 199.5 | 300 | 5.30 | 53.2 |
| 1.001 | 2 | 3 | 19.765 | 0.600 | 34.101 | 34.002 | 0.099 | 199.6 | 300 | 5.59 | 51.8 |
| 1.002 | 3 | 4 | 19.745 | 0.600 | 34.002 | 33.903 | 0.099 | 199.4 | 300 | 5.89 | 50.4 |
| 1.003 | 4 | 5 | 21.885 | 0.600 | 33.903 | 33.768 | 0.135 | 162.1 | 300 | 6.19 | 48.7 |
| 1.004 | 5 | 10 | 42.289 | 0.600 | 33.768 | 33.339 | 0.429 | 98.6 | 300 | 6.63 | 46.7 |
| 1.005 | 10 | 21 | 6.991 | 0.600 | 33.339 | 33.205 | 0.134 | 52.2 | 300 | 6.69 | 46.1 |
| 2.000 | 12 | 16 | 35.329 | 0.600 | 34.000 | 33.765 | 0.235 | 150.3 | 225 | 5.55 | 53.4 |
| 2.001 | 16 | 19 | 46.807 | 0.600 | 33.765 | 33.460 | 0.305 | 153.5 | 225 | 6.29 | 52.1 |
| 3.000 | 17 | 18 | 26.403 | 0.600 | 34.100 | 33.800 | 0.300 | 88.0 | 225 | 5.32 | 48.9 |
| 3.001 | 18 | 19 | 47.753 | 0.600 | 33.800 | 33.450 | 0.350 | 136.4 | 225 | 6.03 | 51.2 |
| 4.000 | New Tank | 19 | 28.687 | 0.600 | 33.625 | 33.450 | 0.175 | 163.9 | 225 | 5.47 | 53.4 |
| 5.000 | 7 | 9 | 35.432 | 0.600 | 34.372 | 34.065 | 0.307 | 115.4 | 150 | 5.63 | 52.0 |
| 5.001 | 9 | 10 | 9.573 | 0.600 | 33.990 | 33.489 | 0.501 | 19.1 | 225 | 5.69 | 51.4 |
| 2.003 | 19 | 21 | 5.695 | 0.600 | 33.300 | 33.205 | 0.095 | 59.9 | 375 | 6.34 | 48.6 |

| Name | US Node | DS Node | Vel (m/s) | Cap (l/s) | Flow (l/s) | US Depth (m) | DS Depth (m) | Σ Area (ha) | Pro Depth (mm) | Pro Velocity (m/s) |
|-------|----------|---------|-----------|-----------|------------|--------------|--------------|-------------|----------------|--------------------|
| 1.000 | 1 | 2 | 1.109 | 78.4 | 5.5 | 0.580 | 0.639 | 0.038 | 54 | 0.647 |
| 1.001 | 2 | 3 | 1.109 | 78.4 | 10.7 | 0.639 | 0.698 | 0.076 | 74 | 0.780 |
| 1.002 | 3 | 4 | 1.109 | 78.4 | 15.6 | 0.698 | 0.797 | 0.114 | 91 | 0.871 |
| 1.003 | 4 | 5 | 1.232 | 87.1 | 20.1 | 0.797 | 0.832 | 0.152 | 98 | 1.006 |
| 1.004 | 5 | 10 | 1.583 | 111.9 | 27.2 | 0.832 | 1.311 | 0.215 | 100 | 1.314 |
| 1.005 | 10 | 21 | 2.181 | 154.2 | 51.0 | 1.311 | 1.345 | 0.408 | 119 | 1.966 |
| 2.000 | 12 | 16 | 1.064 | 42.3 | 6.2 | 0.715 | 0.950 | 0.043 | 58 | 0.767 |
| 2.001 | 16 | 19 | 1.053 | 41.9 | 17.4 | 0.950 | 1.095 | 0.123 | 101 | 1.003 |
| 3.000 | 17 | 18 | 1.394 | 55.4 | 9.0 | 0.775 | 0.875 | 0.068 | 61 | 1.032 |
| 3.001 | 18 | 19 | 1.117 | 44.4 | 16.4 | 0.875 | 1.105 | 0.118 | 94 | 1.035 |
| 4.000 | New Tank | 19 | 1.018 | 40.5 | 11.4 | 1.180 | 1.105 | 0.079 | 81 | 0.876 |
| 5.000 | 7 | 9 | 0.934 | 16.5 | 4.7 | 0.578 | 0.765 | 0.033 | 54 | 0.802 |
| 5.001 | 9 | 10 | 3.007 | 119.6 | 18.5 | 0.765 | 1.236 | 0.133 | 59 | 2.193 |
| 2.003 | 19 | 21 | 2.343 | 258.8 | 55.3 | 1.105 | 1.270 | 0.420 | 117 | 1.877 |



Links

| Name | US Node | DS Node | Length (m) | ks (mm) / n | US IL (m) | DS IL (m) | Fall (m) | Slope (1:X) | Dia (mm) | T of C (mins) | Rain (mm/hr) |
|-------|---------------|---------------|------------|-------------|-----------|-----------|----------|-------------|----------|---------------|--------------|
| 1.006 | 21 | Existing Tank | 3.920 | 0.600 | 33.205 | 33.055 | 0.150 | 26.1 | 375 | 6.70 | 43.5 |
| 1.007 | Existing Tank | 22a | 3.025 | 0.600 | 33.055 | 33.006 | 0.049 | 61.7 | 375 | 6.73 | 42.3 |
| 1.008 | 22a | Outfall | 5.452 | 0.600 | 33.006 | 32.678 | 0.328 | 16.6 | 225 | 6.75 | 43.0 |

| Name | US Node | DS Node | Vel (m/s) | Cap (l/s) | Flow (l/s) | US Depth (m) | DS Depth (m) | Σ Area (ha) | Pro Depth (mm) | Pro Velocity (m/s) |
|-------|---------------|---------------|-----------|-----------|------------|--------------|--------------|-------------|----------------|--------------------|
| 1.006 | 21 | Existing Tank | 3.556 | 392.8 | 100.0 | 1.270 | 1.470 | 0.848 | 129 | 2.992 |
| 1.007 | Existing Tank | 22a | 2.309 | 255.0 | 107.8 | 1.470 | 1.619 | 0.940 | 170 | 2.218 |
| 1.008 | 22a | Outfall | 3.225 | 128.2 | 109.6 | 1.769 | 2.022 | 0.940 | 160 | 3.607 |

Simulation Settings

| | | | | | |
|----------------------|-------------------|-------------------|--------|----------------------------|------|
| Rainfall Methodology | FSR | Summer CV | 0.900 | Drain Down Time (mins) | 1440 |
| FSR Region | England and Wales | Winter CV | 0.900 | Additional Storage (m³/ha) | 0.0 |
| M5-60 (mm) | 20.000 | Analysis Speed | Normal | Check Discharge Rate(s) | x |
| Ratio-R | 0.400 | Skip Steady State | x | Check Discharge Volume | x |

Storm Durations

| | | | | | | | | | | | |
|----|----|----|-----|-----|-----|-----|-----|-----|-----|-----|------|
| 15 | 30 | 60 | 120 | 180 | 240 | 360 | 480 | 600 | 720 | 960 | 1440 |
|----|----|----|-----|-----|-----|-----|-----|-----|-----|-----|------|

| Return Period (years) | Climate Change (CC %) | Additional Area (A %) | Additional Flow (Q %) | Return Period (years) | Climate Change (CC %) | Additional Area (A %) | Additional Flow (Q %) |
|-----------------------|-----------------------|-----------------------|-----------------------|-----------------------|-----------------------|-----------------------|-----------------------|
| 2 | 0 | 0 | 0 | 100 | 20 | 0 | 0 |
| 30 | 0 | 0 | 0 | | | | |

Node 22a Online Hydro-Brake® Control

| | | | |
|--------------------------|--------|-------------------------|-----------------------------|
| Flap Valve | x | Objective | (CL) Minimise blockage risk |
| Replaces Downstream Link | ✓ | Sump Available | ✓ |
| Invert Level (m) | 33.006 | Product Number | CTL-SCL-0142-1110-1050-1110 |
| Design Depth (m) | 1.050 | Min Outlet Diameter (m) | 0.225 |
| Design Flow (l/s) | 11.1 | Min Node Diameter (mm) | 1200 |

Node Existing Tank Depth/Area Storage Structure

| | | | | | |
|-----------------------------|---------|---------------|------|---------------------------|--------|
| Base Inf Coefficient (m/hr) | 0.00000 | Safety Factor | 2.0 | Invert Level (m) | 33.055 |
| Side Inf Coefficient (m/hr) | 0.00000 | Porosity | 0.95 | Time to half empty (mins) | 448 |

| Depth (m) | Area (m ²) | Inf Area (m ²) | Depth (m) | Area (m ²) | Inf Area (m ²) | Depth (m) | Area (m ²) | Inf Area (m ²) |
|--------------|---------------------------|-------------------------------|--------------|---------------------------|-------------------------------|--------------|---------------------------|-------------------------------|
| 0.000 | 380.0 | 0.0 | 1.000 | 380.0 | 0.0 | 1.001 | 0.0 | 0.0 |

Node New Tank Depth/Area Storage Structure

| | | | | | |
|-----------------------------|---------|---------------|------|---------------------------|--------|
| Base Inf Coefficient (m/hr) | 0.00000 | Safety Factor | 2.0 | Invert Level (m) | 33.625 |
| Side Inf Coefficient (m/hr) | 0.00000 | Porosity | 0.95 | Time to half empty (mins) | 228 |

| Depth (m) | Area (m ²) | Inf Area (m ²) | Depth (m) | Area (m ²) | Inf Area (m ²) | Depth (m) | Area (m ²) | Inf Area (m ²) |
|--------------|---------------------------|-------------------------------|--------------|---------------------------|-------------------------------|--------------|---------------------------|-------------------------------|
| 0.000 | 113.0 | 0.0 | 0.610 | 113.0 | 0.0 | 0.611 | 0.0 | 0.0 |

Rainfall

| Event | Peak Intensity (mm/hr) | Average Intensity (mm/hr) | Event | Peak Intensity (mm/hr) | Average Intensity (mm/hr) |
|-------------------------|------------------------------|---------------------------------|--------------------------|------------------------------|---------------------------------|
| 2 year 15 minute summer | 141.566 | 40.058 | 2 year 60 minute winter | 40.727 | 16.200 |
| 2 year 15 minute winter | 99.345 | 40.058 | 2 year 120 minute summer | 37.449 | 9.897 |
| 2 year 30 minute summer | 91.753 | 25.963 | 2 year 120 minute winter | 24.880 | 9.897 |
| 2 year 30 minute winter | 64.388 | 25.963 | 2 year 180 minute summer | 28.672 | 7.378 |
| 2 year 60 minute summer | 61.301 | 16.200 | 2 year 180 minute winter | 18.637 | 7.378 |

Rainfall

| Event | Peak Intensity (mm/hr) | Average Intensity (mm/hr) | Event | Peak Intensity (mm/hr) | Average Intensity (mm/hr) |
|---------------------------|------------------------------|---------------------------------|-------------------------------------|------------------------------|---------------------------------|
| 2 year 240 minute summer | 22.636 | 5.982 | 30 year 600 minute winter | 13.498 | 5.404 |
| 2 year 240 minute winter | 15.039 | 5.982 | 30 year 720 minute summer | 17.490 | 4.687 |
| 2 year 360 minute summer | 17.235 | 4.435 | 30 year 720 minute winter | 11.754 | 4.687 |
| 2 year 360 minute winter | 11.203 | 4.435 | 30 year 960 minute summer | 14.215 | 3.743 |
| 2 year 480 minute summer | 13.550 | 3.581 | 30 year 960 minute winter | 9.416 | 3.743 |
| 2 year 480 minute winter | 9.003 | 3.581 | 30 year 1440 minute summer | 10.161 | 2.723 |
| 2 year 600 minute summer | 11.088 | 3.033 | 30 year 1440 minute winter | 6.829 | 2.723 |
| 2 year 600 minute winter | 7.576 | 3.033 | 100 year +20% CC 15 minute summer | 418.486 | 118.417 |
| 2 year 720 minute summer | 9.878 | 2.647 | 100 year +20% CC 15 minute winter | 293.674 | 118.417 |
| 2 year 720 minute winter | 6.639 | 2.647 | 100 year +20% CC 30 minute summer | 274.758 | 77.747 |
| 2 year 960 minute summer | 8.113 | 2.136 | 100 year +20% CC 30 minute winter | 192.813 | 77.747 |
| 2 year 960 minute winter | 5.374 | 2.136 | 100 year +20% CC 60 minute summer | 183.946 | 48.611 |
| 2 year 1440 minute summer | 5.891 | 1.579 | 100 year +20% CC 60 minute winter | 122.209 | 48.611 |
| 2 year 1440 minute winter | 3.959 | 1.579 | 100 year +20% CC 120 minute summer | 111.074 | 29.354 |
| 30 year 15 minute summer | 268.706 | 76.035 | 100 year +20% CC 120 minute winter | 73.795 | 29.354 |
| 30 year 15 minute winter | 188.566 | 76.035 | 100 year +20% CC 180 minute summer | 83.767 | 21.556 |
| 30 year 30 minute summer | 174.929 | 49.499 | 100 year +20% CC 180 minute winter | 54.451 | 21.556 |
| 30 year 30 minute winter | 122.757 | 49.499 | 100 year +20% CC 240 minute summer | 65.123 | 17.210 |
| 30 year 60 minute summer | 116.589 | 30.811 | 100 year +20% CC 240 minute winter | 43.266 | 17.210 |
| 30 year 60 minute winter | 77.459 | 30.811 | 100 year +20% CC 360 minute summer | 48.580 | 12.501 |
| 30 year 120 minute summer | 70.438 | 18.615 | 100 year +20% CC 360 minute winter | 31.578 | 12.501 |
| 30 year 120 minute winter | 46.797 | 18.615 | 100 year +20% CC 480 minute summer | 37.697 | 9.962 |
| 30 year 180 minute summer | 53.298 | 13.715 | 100 year +20% CC 480 minute winter | 25.045 | 9.962 |
| 30 year 180 minute winter | 34.645 | 13.715 | 100 year +20% CC 600 minute summer | 30.518 | 8.347 |
| 30 year 240 minute summer | 41.604 | 10.995 | 100 year +20% CC 600 minute winter | 20.851 | 8.347 |
| 30 year 240 minute winter | 27.641 | 10.995 | 100 year +20% CC 720 minute summer | 26.943 | 7.221 |
| 30 year 360 minute summer | 31.221 | 8.034 | 100 year +20% CC 720 minute winter | 18.107 | 7.221 |
| 30 year 360 minute winter | 20.295 | 8.034 | 100 year +20% CC 960 minute summer | 21.799 | 5.740 |
| 30 year 480 minute summer | 24.324 | 6.428 | 100 year +20% CC 960 minute winter | 14.440 | 5.740 |
| 30 year 480 minute winter | 16.160 | 6.428 | 100 year +20% CC 1440 minute summer | 15.476 | 4.148 |
| 30 year 600 minute summer | 19.756 | 5.404 | 100 year +20% CC 1440 minute winter | 10.401 | 4.148 |

Results for 2 year Critical Storm Duration. Lowest mass balance: 95.80%

| Node Event | US Node | Peak (mins) | Level (m) | Depth (m) | Inflow (l/s) | Node Vol (m³) | Flood (m³) | Status |
|------------------|----------|-------------|-----------|-----------|--------------|---------------|------------|--------|
| 15 minute summer | 1 | 10 | 34.263 | 0.063 | 7.9 | 0.0548 | 0.0000 | OK |
| 15 minute summer | 2 | 10 | 34.193 | 0.092 | 15.7 | 0.0796 | 0.0000 | OK |
| 15 minute summer | 3 | 11 | 34.120 | 0.118 | 23.3 | 0.1021 | 0.0000 | OK |
| 15 minute summer | 4 | 11 | 34.033 | 0.130 | 30.7 | 0.1126 | 0.0000 | OK |
| 15 minute summer | 5 | 11 | 33.897 | 0.129 | 43.4 | 0.1115 | 0.0000 | OK |
| 15 minute summer | 10 | 11 | 33.534 | 0.195 | 82.1 | 0.1686 | 0.0000 | OK |
| 15 minute summer | 12 | 10 | 34.069 | 0.069 | 8.9 | 0.0597 | 0.0000 | OK |
| 15 minute summer | 16 | 11 | 33.892 | 0.127 | 25.3 | 0.1100 | 0.0000 | OK |
| 60 minute summer | New Tank | 38 | 33.681 | 0.056 | 10.9 | 5.9880 | 0.0000 | OK |
| 15 minute summer | 17 | 10 | 34.177 | 0.077 | 14.1 | 0.0663 | 0.0000 | OK |
| 15 minute summer | 18 | 11 | 33.920 | 0.120 | 24.4 | 0.1036 | 0.0000 | OK |
| 15 minute summer | 7 | 11 | 34.440 | 0.068 | 6.9 | 0.0586 | 0.0000 | OK |
| 15 minute summer | 9 | 10 | 34.070 | 0.080 | 27.4 | 0.0690 | 0.0000 | OK |
| 15 minute summer | 19 | 11 | 33.474 | 0.174 | 70.7 | 0.2489 | 0.0000 | OK |

| Link Event (Upstream Depth) | US Node | Link | DS Node | Outflow (l/s) | Velocity (m/s) | Flow/Cap | Link Vol (m³) |
|-----------------------------|----------|-------|---------|---------------|----------------|----------|---------------|
| 15 minute summer | 1 | 1.000 | 2 | 7.8 | 0.541 | 0.100 | 0.2871 |
| 15 minute summer | 2 | 1.001 | 3 | 15.4 | 0.705 | 0.197 | 0.4335 |
| 15 minute summer | 3 | 1.002 | 4 | 23.2 | 0.845 | 0.296 | 0.5422 |
| 15 minute summer | 4 | 1.003 | 5 | 31.0 | 1.065 | 0.355 | 0.6361 |
| 15 minute summer | 5 | 1.004 | 10 | 43.6 | 1.127 | 0.390 | 1.6337 |
| 15 minute summer | 10 | 1.005 | 21 | 82.4 | 1.743 | 0.534 | 0.3304 |
| 15 minute summer | 12 | 2.000 | 16 | 8.8 | 0.550 | 0.207 | 0.5878 |
| 15 minute summer | 16 | 2.001 | 19 | 24.6 | 1.089 | 0.589 | 1.0591 |
| 60 minute summer | New Tank | 4.000 | 19 | 5.4 | 0.713 | 0.134 | 0.2177 |
| 15 minute summer | 17 | 3.000 | 18 | 14.0 | 0.877 | 0.252 | 0.4372 |
| 15 minute summer | 18 | 3.001 | 19 | 23.8 | 1.131 | 0.536 | 1.0051 |
| 15 minute summer | 7 | 5.000 | 9 | 6.7 | 0.882 | 0.407 | 0.2699 |
| 15 minute summer | 9 | 5.001 | 10 | 27.3 | 2.307 | 0.228 | 0.1133 |
| 15 minute summer | 19 | 2.003 | 21 | 70.7 | 1.349 | 0.273 | 0.2986 |



Results for 2 year Critical Storm Duration. Lowest mass balance: 95.80%

| Node Event | US Node | Peak (mins) | Level (m) | Depth (m) | Inflow (l/s) | Node Vol (m³) | Flood (m³) | Status |
|-------------------|---------------|-------------|-----------|-----------|--------------|---------------|------------|------------|
| 15 minute summer | 21 | 11 | 33.392 | 0.187 | 157.1 | 0.2675 | 0.0000 | OK |
| 120 minute summer | Existing Tank | 102 | 33.360 | 0.305 | 98.5 | 110.0424 | 0.0000 | OK |
| 120 minute summer | 22a | 102 | 33.360 | 0.354 | 41.9 | 0.6252 | 0.0000 | SURCHARGED |
| 15 minute summer | Outfall | 1 | 32.678 | 0.000 | 11.1 | 0.0000 | 0.0000 | OK |

| Link Event (Upstream Depth) | US Node | Link | DS Node | Outflow (l/s) | Velocity (m/s) | Flow/Cap | Link Vol (m³) |
|-----------------------------|---------------|--------------|---------------|---------------|----------------|----------|---------------|
| 15 minute summer | 21 | 1.006 | Existing Tank | 157.0 | 4.764 | 0.400 | 0.1725 |
| 120 minute summer | Existing Tank | 1.007 | 22a | 41.9 | 0.749 | 0.164 | 0.3082 |
| 120 minute summer | 22a | Hydro-Brake® | Outfall | 11.1 | | | |

Results for 30 year Critical Storm Duration. Lowest mass balance: 95.80%

| Node Event | US Node | Peak (mins) | Level (m) | Depth (m) | Inflow (l/s) | Node Vol (m³) | Flood (m³) | Status |
|-------------------|----------|-------------|-----------|-----------|--------------|---------------|------------|------------|
| 15 minute summer | 1 | 10 | 34.288 | 0.088 | 15.0 | 0.0758 | 0.0000 | OK |
| 15 minute summer | 2 | 10 | 34.237 | 0.136 | 29.9 | 0.1176 | 0.0000 | OK |
| 15 minute summer | 3 | 11 | 34.179 | 0.177 | 44.4 | 0.1536 | 0.0000 | OK |
| 15 minute summer | 4 | 11 | 34.101 | 0.198 | 58.5 | 0.1718 | 0.0000 | OK |
| 15 minute summer | 5 | 11 | 33.977 | 0.209 | 82.2 | 0.1809 | 0.0000 | OK |
| 15 minute summer | 10 | 11 | 33.757 | 0.418 | 154.4 | 0.3617 | 0.0000 | SURCHARGED |
| 15 minute summer | 12 | 10 | 34.098 | 0.098 | 17.0 | 0.0845 | 0.0000 | OK |
| 15 minute summer | 16 | 11 | 33.999 | 0.234 | 48.4 | 0.2027 | 0.0000 | SURCHARGED |
| 180 minute winter | New Tank | 180 | 33.727 | 0.102 | 6.7 | 10.9224 | 0.0000 | OK |
| 15 minute summer | 17 | 10 | 34.209 | 0.109 | 26.9 | 0.0946 | 0.0000 | OK |
| 15 minute summer | 18 | 11 | 33.997 | 0.197 | 46.4 | 0.1705 | 0.0000 | OK |
| 15 minute summer | 7 | 10 | 34.473 | 0.101 | 13.0 | 0.0876 | 0.0000 | OK |
| 15 minute summer | 9 | 10 | 34.101 | 0.111 | 52.0 | 0.0958 | 0.0000 | OK |
| 180 minute winter | 19 | 180 | 33.728 | 0.428 | 35.0 | 0.6122 | 0.0000 | SURCHARGED |

| Link Event (Upstream Depth) | US Node | Link | DS Node | Outflow (l/s) | Velocity (m/s) | Flow/Cap | Link Vol (m³) |
|-----------------------------|----------|-------|---------|---------------|----------------|----------|---------------|
| 15 minute summer | 1 | 1.000 | 2 | 14.9 | 0.623 | 0.190 | 0.4744 |
| 15 minute summer | 2 | 1.001 | 3 | 29.4 | 0.791 | 0.374 | 0.7331 |
| 15 minute summer | 3 | 1.002 | 4 | 44.0 | 0.951 | 0.561 | 0.9163 |
| 15 minute summer | 4 | 1.003 | 5 | 58.5 | 1.203 | 0.672 | 1.1143 |
| 15 minute summer | 5 | 1.004 | 10 | 81.4 | 1.233 | 0.727 | 2.5966 |
| 15 minute summer | 10 | 1.005 | 21 | 154.2 | 2.190 | 1.000 | 0.4921 |
| 15 minute summer | 12 | 2.000 | 16 | 16.8 | 0.622 | 0.397 | 0.9891 |
| 15 minute summer | 16 | 2.001 | 19 | 44.5 | 1.217 | 1.063 | 1.7125 |
| 180 minute winter | New Tank | 4.000 | 19 | 6.3 | 0.740 | 0.156 | 0.8204 |
| 15 minute summer | 17 | 3.000 | 18 | 26.7 | 1.002 | 0.482 | 0.7321 |
| 15 minute summer | 18 | 3.001 | 19 | 44.8 | 1.270 | 1.007 | 1.6796 |
| 15 minute summer | 7 | 5.000 | 9 | 12.7 | 1.020 | 0.767 | 0.4395 |
| 15 minute summer | 9 | 5.001 | 10 | 51.8 | 2.474 | 0.433 | 0.2833 |
| 180 minute winter | 19 | 2.003 | 21 | 40.3 | 1.026 | 0.156 | 0.6281 |



Results for 30 year Critical Storm Duration. Lowest mass balance: 95.80%

| Node Event | US Node | Peak (mins) | Level (m) | Depth (m) | Inflow (l/s) | Node Vol (m³) | Flood (m³) | Status |
|-------------------|---------------|-------------|-----------|-----------|--------------|---------------|------------|------------|
| 180 minute winter | 21 | 180 | 33.726 | 0.521 | 76.3 | 0.7461 | 0.0000 | SURCHARGED |
| 180 minute winter | Existing Tank | 176 | 33.726 | 0.671 | 95.2 | 242.3841 | 0.0000 | SURCHARGED |
| 180 minute winter | 22a | 176 | 33.726 | 0.720 | 46.1 | 1.2720 | 0.0000 | SURCHARGED |
| 15 minute summer | Outfall | 1 | 32.678 | 0.000 | 11.1 | 0.0000 | 0.0000 | OK |

| Link Event (Upstream Depth) | US Node | Link | DS Node | Outflow (l/s) | Velocity (m/s) | Flow/Cap | Link Vol (m³) |
|-----------------------------|---------------|--------------|---------------|---------------|----------------|----------|---------------|
| 180 minute winter | 21 | 1.006 | Existing Tank | 76.1 | 2.526 | 0.194 | 0.4324 |
| 180 minute winter | Existing Tank | 1.007 | 22a | 46.1 | 0.738 | 0.181 | 0.3336 |
| 180 minute winter | 22a | Hydro-Brake® | Outfall | 11.1 | | | |

Results for 100 year +20% CC Critical Storm Duration. Lowest mass balance: 95.80%

| Node Event | US Node | Peak (mins) | Level (m) | Depth (m) | Inflow (l/s) | Node Vol (m³) | Flood (m³) | Status |
|-------------------|----------|-------------|-----------|-----------|--------------|---------------|------------|------------|
| 15 minute summer | 1 | 12 | 34.993 | 0.793 | 23.4 | 0.6867 | 0.0000 | FLOOD RISK |
| 15 minute summer | 2 | 12 | 34.984 | 0.883 | 43.7 | 0.7648 | 0.0000 | FLOOD RISK |
| 15 minute summer | 3 | 12 | 34.949 | 0.947 | 62.9 | 0.8205 | 0.0000 | FLOOD RISK |
| 15 minute summer | 4 | 12 | 34.872 | 0.969 | 82.6 | 0.8387 | 0.0000 | FLOOD RISK |
| 15 minute summer | 5 | 12 | 34.721 | 0.953 | 110.6 | 0.8251 | 0.0000 | FLOOD RISK |
| 15 minute summer | 10 | 12 | 34.211 | 0.872 | 216.5 | 0.7548 | 0.0000 | SURCHARGED |
| 15 minute summer | 12 | 11 | 34.811 | 0.811 | 26.5 | 0.7024 | 0.0000 | FLOOD RISK |
| 15 minute summer | 16 | 11 | 34.724 | 0.959 | 70.0 | 0.8307 | 0.0000 | FLOOD RISK |
| 240 minute winter | New Tank | 184 | 34.060 | 0.435 | 20.2 | 46.7275 | 0.0000 | SURCHARGED |
| 15 minute summer | 17 | 11 | 34.846 | 0.746 | 41.8 | 0.6463 | 0.0000 | FLOOD RISK |
| 15 minute summer | 18 | 11 | 34.673 | 0.873 | 67.4 | 0.7559 | 0.0000 | FLOOD RISK |
| 15 minute summer | 7 | 12 | 34.877 | 0.505 | 20.3 | 0.4372 | 0.0000 | FLOOD RISK |
| 15 minute summer | 9 | 11 | 34.495 | 0.505 | 77.7 | 0.4374 | 0.0000 | SURCHARGED |
| 120 minute winter | 19 | 116 | 34.068 | 0.768 | 74.8 | 1.0985 | 0.0000 | SURCHARGED |

| Link Event (Upstream Depth) | US Node | Link | DS Node | Outflow (l/s) | Velocity (m/s) | Flow/Cap | Link Vol (m³) |
|-----------------------------|----------|-------|---------|---------------|----------------|----------|---------------|
| 15 minute summer | 1 | 1.000 | 2 | 21.4 | 0.658 | 0.273 | 1.3909 |
| 15 minute summer | 2 | 1.001 | 3 | 44.8 | 0.815 | 0.572 | 1.3918 |
| 15 minute summer | 3 | 1.002 | 4 | 60.3 | 0.931 | 0.768 | 1.3904 |
| 15 minute summer | 4 | 1.003 | 5 | 79.4 | 1.175 | 0.912 | 1.5411 |
| 15 minute summer | 5 | 1.004 | 10 | 111.7 | 1.586 | 0.998 | 2.9780 |
| 15 minute summer | 10 | 1.005 | 21 | 214.8 | 3.051 | 1.393 | 0.4923 |
| 15 minute summer | 12 | 2.000 | 16 | 24.0 | 0.669 | 0.568 | 1.4051 |
| 15 minute summer | 16 | 2.001 | 19 | 68.2 | 1.716 | 1.630 | 1.8616 |
| 240 minute winter | New Tank | 4.000 | 19 | -12.5 | 0.750 | -0.308 | 1.1409 |
| 15 minute summer | 17 | 3.000 | 18 | 38.1 | 1.078 | 0.686 | 1.0501 |
| 15 minute summer | 18 | 3.001 | 19 | 65.7 | 1.652 | 1.479 | 1.8992 |
| 15 minute summer | 7 | 5.000 | 9 | 18.0 | 1.045 | 1.091 | 0.6238 |
| 15 minute summer | 9 | 5.001 | 10 | 75.1 | 2.455 | 0.628 | 0.3807 |
| 120 minute winter | 19 | 2.003 | 21 | 71.6 | 1.079 | 0.277 | 0.6281 |



Results for 100 year +20% CC Critical Storm Duration. Lowest mass balance: 95.80%

| Node Event | US Node | Peak (mins) | Level (m) | Depth (m) | Inflow (l/s) | Node Vol (m³) | Flood (m³) | Status |
|-------------------|---------------|-------------|-----------|-----------|--------------|---------------|------------|------------|
| 120 minute winter | 21 | 116 | 34.068 | 0.863 | 147.9 | 1.2353 | 0.0000 | SURCHARGED |
| 240 minute summer | Existing Tank | 232 | 34.057 | 1.002 | 144.6 | 361.1805 | 0.0000 | SURCHARGED |
| 180 minute winter | 22a | 164 | 34.057 | 1.051 | 52.7 | 1.8576 | 0.0000 | SURCHARGED |
| 15 minute summer | Outfall | 1 | 32.678 | 0.000 | 11.1 | 0.0000 | 0.0000 | OK |

| Link Event (Upstream Depth) | US Node | Link | DS Node | Outflow (l/s) | Velocity (m/s) | Flow/Cap | Link Vol (m³) |
|-----------------------------|---------------|--------------|---------------|---------------|----------------|----------|---------------|
| 120 minute winter | 21 | 1.006 | Existing Tank | 147.2 | 3.194 | 0.375 | 0.4324 |
| 240 minute summer | Existing Tank | 1.007 | 22a | 39.3 | 0.714 | 0.154 | 0.3336 |
| 180 minute winter | 22a | Hydro-Brake® | Outfall | 11.1 | | | |

APPENDIX H SUDS PROFORMA

| | | |
|---------------------------|---|---|
| 1. Project & Site Details | Project / Site Name (including sub-catchment / stage / phase where appropriate) | Lidl South Ruislip |
| | Address & post code | Victoria Road, Ruislip HA4 0QQ |
| | OS Grid ref. (Easting, Northing) | E 512139.0986 N 185590.0634 |
| | LPA reference (if applicable) | |
| | Brief description of proposed work | Demolition of the existing building Bensons for Beds Building and use the space for additional car parking |
| | Total site Area | 9400 m ² |
| | Total existing impervious area | 9400 m ² |
| | Total proposed impervious area | 9400 m ² |
| | Is the site in a surface water flood risk catchment (ref. local Surface Water Management Plan)? | The site is situated in an area of low surface water flood risk. Please refer to the document attached to the proforma. |
| | Existing drainage connection type and location | Pipe connection to existing culvert. Location shown on drawing RSK-C-0003 Drainage Strategy |
| Designer Name | Assif Salim | |

| | | | |
|--|--|-----------------------|----------------|
| 2. Proposed Discharge Arrangements | 2a. Infiltration Feasibility | | |
| | Superficial geology classification | London Clay | |
| | Bedrock geology classification | London Clay formation | |
| | Site infiltration rate | m/s | |
| | Depth to groundwater level | m below ground level | |
| | Is infiltration feasible? | Not possible | |
| | 2b. Drainage Hierarchy | | |
| | | Feasible (Y/N) | Proposed (Y/N) |
| | 1 store rainwater for later use | N | N |
| | 2 use infiltration techniques, such as porous surfaces in non-clay areas | N | N |
| | 3 attenuate rainwater in ponds or open water features for gradual release | N | N |
| | 4 attenuate rainwater by storing in tanks or sealed water features for gradual release | Y | Y |
| | 5 discharge rainwater direct to a watercourse | N | N |
| | 6 discharge rainwater to a surface water sewer/drain | Y | Y |
| 7 discharge rainwater to the combined sewer. | N | N | |
| 2c. Proposed Discharge Details | | | |
| Proposed discharge location | See drawing RSK-C-0003 Drainage Strategy | | |
| Has the owner/regulator of the | The existing offsite drainage connection will | | |

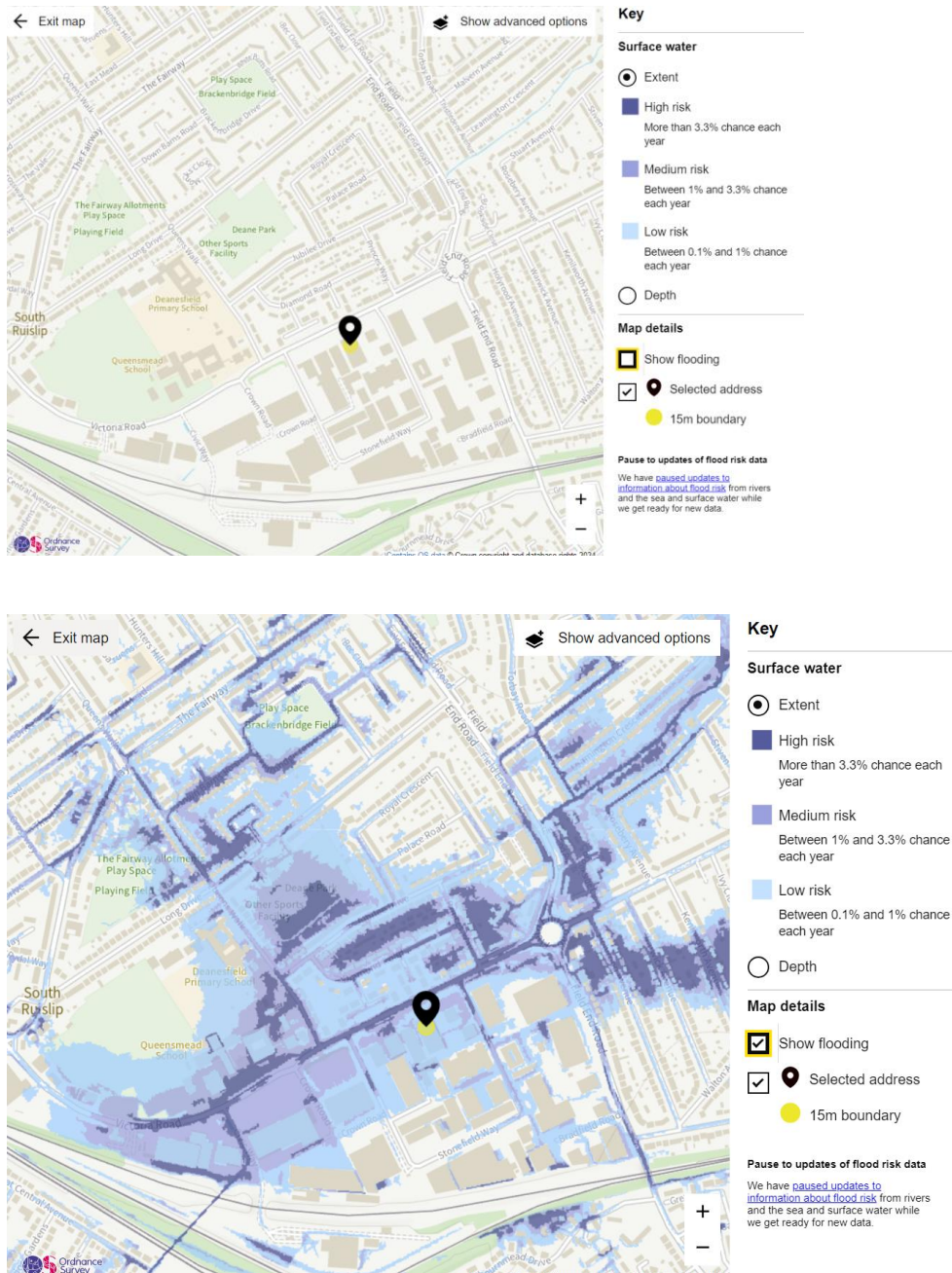
| | | | | | |
|--------------------------|---|-----------------------------------|--------------------------------------|--|-------------------------------|
| 3. Drainage Strategy | Designer Position | | Senior Civil Engineer | | |
| | Designer Company | | RSK Land and Development Engineering | | |
| | 3a. Discharge Rates & Required Storage | | | | |
| | | Greenfield (GF) runoff rate (l/s) | Existing discharge rate (l/s) | Required storage for GF rate (m ³) | Proposed discharge rate (l/s) |
| | Q _{bar} | | | | |
| | 1 in 1 | | 11 | 426 | 11 |
| | 1 in 30 | | 11 | 426 | 11 |
| | 1 in 100 | | 11 | 426 | 11 |
| | 1 in 100 + CC | | | 426 | 11 |
| | Climate change allowance used | | 20% | | |
| | 3b. Principal Method of Flow Control | | Vortex Flow Control Unit | | |
| | 3c. Proposed SuDS Measures | | | | |
| | | Catchment area (m ²) | Plan area (m ³) | Storage vol. (m ³) | |
| | Rainwater harvesting | 0 | | 0 | |
| | Infiltration systems | 0 | | 0 | |
| Green roofs | 0 | 0 | 0 | | |
| Blue roofs | 0 | 0 | 0 | | |
| Filter strips | 0 | 0 | 0 | | |
| Filter drains | 0 | 0 | 0 | | |
| Bioretention / tree pits | 0 | 0 | 0 | | |
| Pervious pavements | 0 | 0 | 0 | | |
| Swales | 0 | 0 | 0 | | |

| | | |
|--|---|---|
| discharge location been consulted? | | be maintained, with no increase of surface water flows. |
| 4. Supporting Information | 4a. Discharge & Drainage Strategy | <i>Page/section of drainage report</i> |
| | Infiltration feasibility (2a) – geotechnical factual and interpretive reports, including infiltration results | Section 2.2 Offsite Discharge Options |
| | Drainage hierarchy (2b) | Section 2.2 Offsite Discharge Options |
| | Proposed discharge details (2c) – utility plans, correspondence / approval from owner/regulator of discharge location | N/A - No changes to offsite connection are proposed. No increase in flow rates. |
| | Discharge rates & storage (3a) – detailed hydrologic and hydraulic calculations | Section 2.3 - Refer to Drainage calculations in Appendix G |
| | Proposed SuDS measures & specifications (3b) | |
| | 4b. Other Supporting Details | <i>Page/section of drainage report</i> |
| | Detailed Development Layout | Please see Appendix C |
| | Detailed drainage design drawings, including exceedance flow routes | Please see Appendix F |
| | Detailed landscaping plans | Soft landscaping plan in Appendix C |
| Maintenance strategy | Please see Appendix I | |
| Demonstration of how the proposed SuDS measures improve: | Water quality achieved by existing bypass separator | |

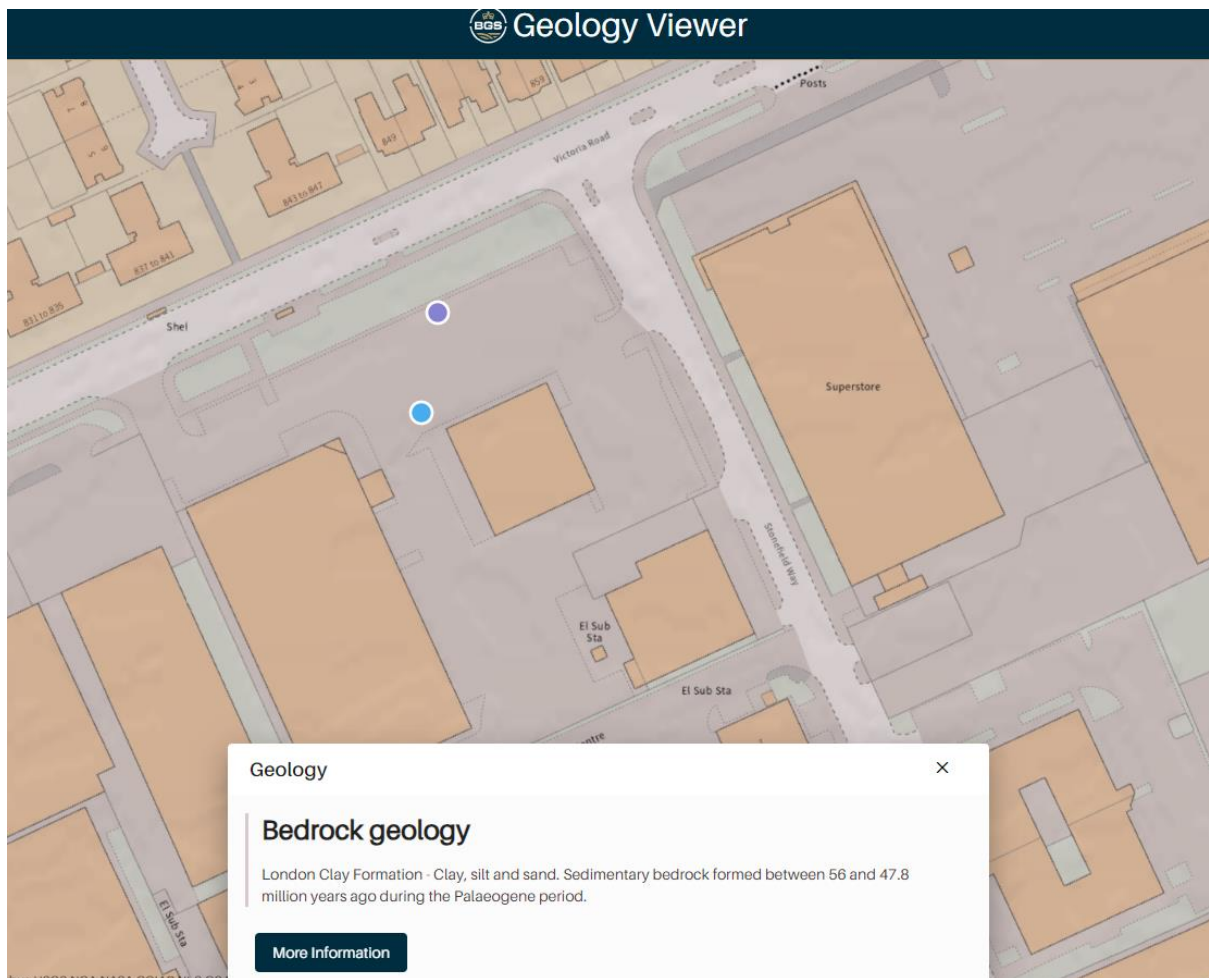
| | | | |
|-------------------|-------------|----------|------------|
| Basins/ponds | 0 | 0 | 0 |
| Attenuation tanks | 9400 | | 426 |
| Total | 9400 | 0 | 426 |

| | |
|---------------------------------|--|
| a) water quality of the runoff? | |
| b) biodiversity? | |
| c) amenity? | |

Assessment of Surface Water Flood Risk – [Where do you want to check? - Check your long term flood risk - GOV.UK \(check-long-term-flood-risk.service.gov.uk\)](https://check-long-term-flood-risk.service.gov.uk)



Geological Information



APPENDIX I

DETAILED MAINTENANCE SCHEDULE



LIDL GB

DEMOLITION OF A NON-FOOD RETAIL UNIT AND EXTENSION AND RECONFIGURATION OF THE EXISTING CAR PARK

Sustainable Drainage System (SuDS) Management Strategy

Project No.890564



24TH APRIL 2024



RSK GENERAL NOTES

Project No.: 890564




Title: SuDS Maintenance Schedule

Client: LiDL GB

Date: 24th April 2024

Office: RSK Land and Development Ltd, Coventry, Abbey Park, Humber Road, CV3 4AQ

Status: Final

| | | | |
|------------------------|---|---------------------------|---|
| Author |  | Technical reviewer |  |
| | | | |
| Date: | 24/04/2024 | Date: | 24/04/2024 |
| Project manager |  | | |
| | | | |
| Date: | 24/04/2024 | | |

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Where field investigations have been carried out, these have been restricted to a level of detail required to achieve the stated objectives of the work.

This work has been undertaken in accordance with the quality management system of RSK LDE Ltd.

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1 INTRODUCTION

This management strategy has been prepared by RSK Land and Development Engineering Ltd on behalf of *LIDL GB*, to support a planning application to the development.

The SUDS considered for the purposes of this statement, include drainage features that will be employed to reduce and manage surface water runoff from the development to a design return period of One hundred years plus climate change. This is required so that The Development will not increase the risk of flooding to the site and its environs. All drainage on site is taken to the underlying strata via infiltration features. Such features include the following:

- Geocellular Tank
- Existing Proprietary treatment system

This document outlines the long-term maintenance of the proposed surface water system and will refer to the following documents, some of which provide further detail on the maintenance operations required:

- CIRIA Report C753, *'The SUDS Manual'*, 2015
- CIRIA Report C625, *'Model Agreements for Sustainable Water Management Systems'*, 2004; and
- Interpave, *'Permeable pavements: Guide to the Design, Construction and Maintenance of Concrete Block Permeable Pavements'*, ed. 4, 2006.

2 MAINTENANCE RESPONSIBILITIES

Responsibility for drainage within England and Wales rests with various bodies. For The Development, the drainage responsibilities will be divided between the following:

- **Private Landowner / Management Company** – The landowner/Management Company will be responsible for the maintenance of drainage features within the Site to maintain all the new site drainage features such as the geocellular attenuation tank, road gullies and bypass separator.

3 MAINTENANCE REGIME

As the maintenance of the communal SuDS features will be carried out via a Management Company, the form of agreement should include the required maintenance listed below. Should the maintenance be transferred at a later date to a public body, then the model agreement SuDS MA1 should be used, details of which can be found in the CIRIA guidance C625.

The following section describes the required maintenance for each feature in turn. The SuDS maintenance requirements listed below should be reviewed after the first 5 years, with a view to agreeing a new regime for the ongoing maintenance.

Notwithstanding the routine inspections and maintenance requirements, after severe storm events all features shall be inspected to clear debris and repair damaged structures or features. Records of the maintenance carried out shall be prepared by the Management Company.

3.1 Storage Tanks

| Maintenance schedule | Required action | Typical frequency |
|----------------------|--|------------------------------------|
| Regular maintenance | Inspect and identify any areas that are not operating correctly. If required, take remedial action | Monthly for 3 months, the annually |
| | Remove debris from the catchment surface (where it may cause risks to performance) | Monthly |
| | For systems where rainfall infiltrates into the tank from above, check surface of filter for blockage by sediment, algae or other matter; remove and replace surface infiltration medium as necessary. | Annually |
| | Remove sediment from pre-treatment structures and/or internal forebays | Annually, or as required |
| Remedial actions | Repair/rehabilitate inlets, outlet, overflows and vents | As required |
| Monitoring | Inspect/check all inlets, outlets, vents and overflows to ensure that they are in good condition and operating as designed | Annually |
| | Survey inside of tank for sediment build-up and remove if necessary | Every 5 years or as required |

3.2 An example of operation and maintenance requirements for a proprietary treatment system

| Maintenance schedule | Required action | Typical frequency |
|----------------------|--|--------------------------------|
| Routine maintenance | Remove litter and debris and inspect for sediment, oil and grease accumulation | Six monthly |
| | Change the filter media | As recommended by manufacturer |

| Maintenance schedule | Required action | Typical frequency |
|----------------------|---|---|
| | Remove sediment, oil, grease and floatables | As necessary – indicated by system inspections or immediately following significant spill |
| Remedial actions | Replace malfunctioning parts or structures | As required |
| Monitoring | Inspect for evidence of poor operation | Six monthly |
| | Inspect filter media and establish appropriate replacement frequencies | Six monthly |
| | Inspect sediment accumulation rates and establish appropriate removal frequencies | Monthly during first half year of operation, then every six months |

3.3 Vortex Flow Control Manhole

| Maintenance schedule | Required action | Typical frequency |
|------------------------|--|-----------------------------|
| Occasional maintenance | Remove litter, debris, and detritus that enters the drainage system | Six monthly, or as required |
| | Inspect inlets, outlets and overflows for blockages, and clear if required | Six monthly, or as required |
| | Inspection Hydrobrake bypass door, open and close door, greasing of door to prevent mechanism from seizing | Six monthly, or as required |
| Remedial actions | Replace components of Hydrobrake if faulty in line with manufacturers guidance. | As required |
| | Replace entire Hydrobrake if components cannot be replaced in accordance with manufacturers guidance. | As required |

APPENDIX A

INSPECTION CHECKLIST

| General information | | | |
|---|--|--|--|
| Site ID | | | |
| Site location and co-ordinates (GIS if appropriate) | | | |
| Elements forming the SuDS scheme | | Approved drawing reference | |
| Inspection frequency | | Approved specification reference | |
| Type of development | | Specific purpose of any parts of the scheme (e.g. biodiversity, wildlife and visual aspects) | |

| Inspection Date | Details | Y/ N | Action required | Date completed | Details | Y/ N | Action required | Date completed |
|--|---------|---------|-----------------|----------------|---------|---------|-----------------|----------------|
| General inspection items | | | | | | | | |
| Is there any evidence of erosion, channelling, ponding (where not desirable) or other poor hydraulic performance? | | | | | | | | |
| Is there any evidence of accidental spillages, oils, poor water quality, odours or nuisance insects? | | | | | | | | |
| Have any health and safety risks been identified to either the public or maintenance operatives? | | | | | | | | |
| Silt/Sediment accumulation | | | | | | | | |
| Is there any sediment accumulation at inlets (or other defined accumulation zones such as the surface of filter drains or infiltration basins and within proprietary devices)? If yes, state depth (mm) and extent. Is removal required? If yes, state waste disposal requirements and confirm that all waste management requirements have been complied with (consult environmental regulator) | | | | | | | | |
| Is surface clogging visible potentially problematic where water has to soak into the underlying construction or ground (e.g. underdrained swale or infiltration basin)? | | | | | | | | |

| Inspection Date | Details | Y/ N | Action required | Date completed | Details | Y/ N | Action required | Date completed |
|-----------------|---|---------|-----------------|----------------|---------|---------|-----------------|----------------|
| | Does permeable or porous surfacing require sweeping to remove silt? | | | | | | | |
| | Is there evidence of litter accumulation in the system? If yes, is this a blockage risk? | | | | | | | |
| | Is there any evidence of any other clogging or blockage of outlets or drainage paths? | | | | | | | |
| | Is the vegetation condition satisfactory (density, weed growth, coverage etc)? (check against approved planting regime) | | | | | | | |
| | Does any part of the system require weeding, pruning or mowing? (check against maintenance frequency state in approved design). | | | | | | | |
| | Is there any evidence of invasive species becoming established? If yes, state action required | | | | | | | |
| | Are any check dams or weirs in good condition? | | | | | | | |
| | Is there any evidence of any accidental damage to the system (e.g. wheel ruts?) | | | | | | | |
| | Is there any evidence of cross connections or other unauthorised inflows? | | | | | | | |
| | Is there any evidence of tampering with the flow control? | | | | | | | |
| | Are there any other matters that could affect the performance of the | | | | | | | |

| Inspection Date | Details | Y/ N | Action required | Date completed | Details | Y/ N | Action required | Date completed |
|--|---------|---------|-----------------|-------------------|---------|---------|-----------------|-------------------|
| system in relation to the design objectives for hydraulic, water quality, biodiversity and visual aspects? | | | | | | | | |
| Other observations | | | | | | | | |
| Information appended (e.g. photos) | | | | | | | | |
| Continue as current Increase maintenance Decrease maintenance | | | | | | | | |
| Proposed date for next inspection | | | | | | | | |