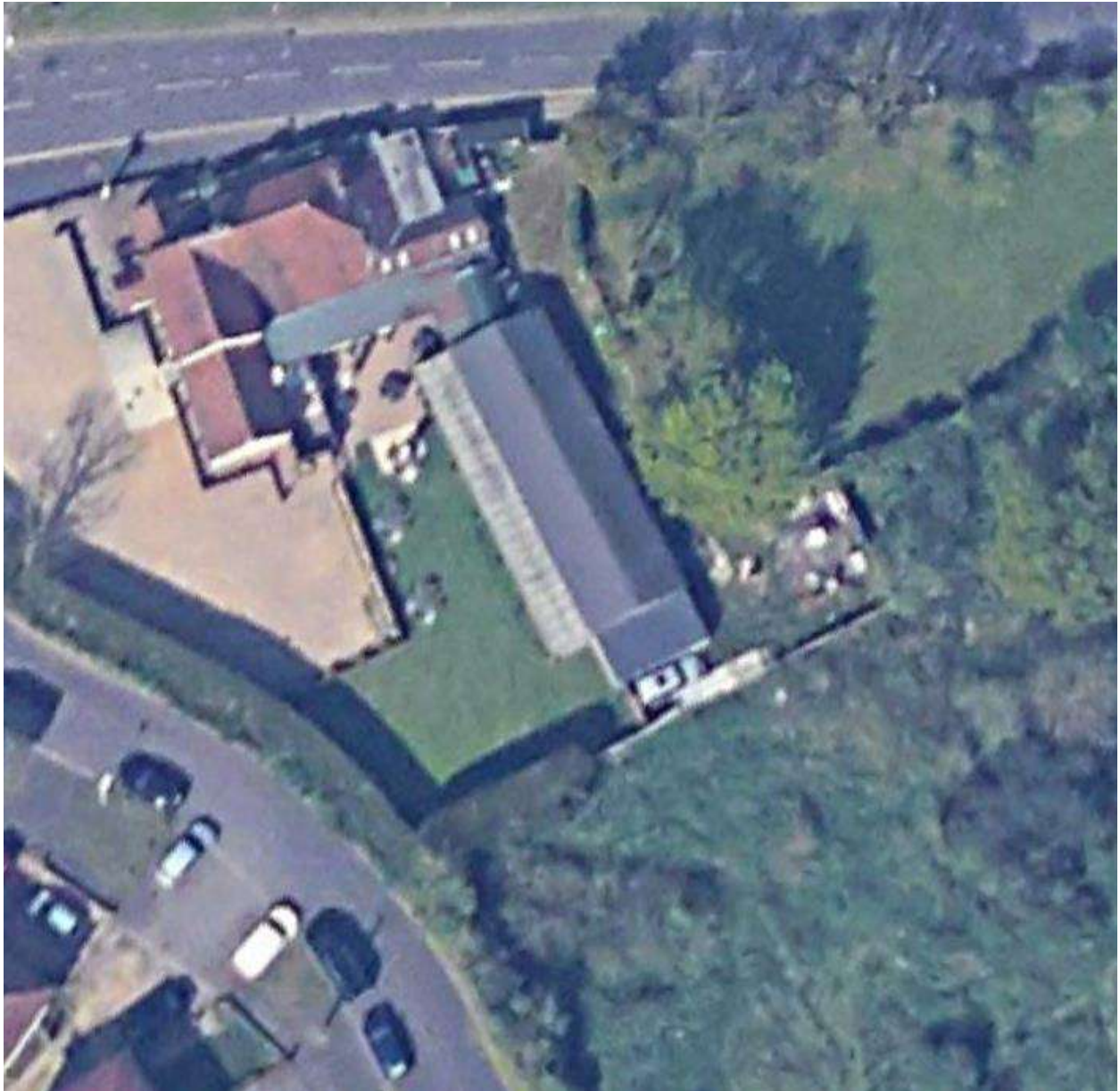


## Soakaway Testing

Land to Rear of The Hut Pub, Old Orchard Cl, Uxbridge



Imagery © 2025 Google Earth.

Contract No./Report Type: 25-045/ SA  
Produced for White House Design


# AG Geo-Consultants Ltd

Report Title
Soakaway Testing

Project	Project No./Report Type
Land to Rear of The Hut Pub, Old Orchard Cl, Uxbridge	25-045/SA

Client Name
White House Design

Issue Date/Version	Status	Comments, Description of Amendments
18 <sup>th</sup> June 2025	Final	-

Report Prepared by:			
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*Specialist in Land Condition (SiLC) No. A1201    Qualified Person (QP), No.086 (CL:AIRE Definition of Waste, Industry Code of Practice, DoWCoP)*

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# AG Geo-Consultants Ltd

Soakaway Testing  
Land to Rear of The Hut Pub, Old Orchard Cl, Uxbridge

## Contents

<b>Executive Summary .....</b>	<b>2</b>
<b>1 Introduction .....</b>	<b>3</b>
1.1 Introduction and Brief .....	3
1.2 Proposed Development.....	3
1.3 Scope of Works.....	3
1.4 Limitations.....	3
<b>2 Phase 1 Desk Study .....</b>	<b>5</b>
2.1 Introduction.....	5
2.2 Desk Study.....	5
<b>3 Phase 2 Site Investigation .....</b>	<b>6</b>
3.1 General .....	6
3.2 Trial Pitting .....	6
3.3 In-Situ Testing.....	6
<b>4 Ground Conditions.....</b>	<b>7</b>
4.1 General .....	7
4.2 Groundwater .....	7
4.3 Excavations.....	7
4.4 Contamination Indications .....	8
<b>5 Soakage Findings .....</b>	<b>9</b>

## Appendices

# AG Geo-Consultants Ltd

Soakaway Testing  
Land to Rear of The Hut Pub, Old Orchard Cl, Uxbridge

## Executive Summary

Client	White House Design																
Site and Location	Land to Rear of The Hut Pub, Old Orchard Cl, Uxbridge Approximate postcode = UB8 3LH																
Proposed Development	Detached house.																
Client Brief	Suds drainage infiltration testing																
History of Site & Surroundings	History not checked. Currently .... <b>On Site Conditions</b> ½ (west) pub beer garden and ½ (east) storage and waste ground area. <b>In the Surroundings</b> Small farm, country park, residential areas.																
Ground Conditions	<div><div><div>BGS Mapping Suggests:</div><div><div><div>• <b>Drift Deposits:</b> Boyn Hill Gravel Member - Sand and gravel.</div><div>• <b>Solid Geology:</b> London Clay Formation - Clay, silt and sand.</div><div>• <b>The nearest relevant BGS boreholes (on the same geology) suggest:</b><div><div>- 0m-6.5m: very dense SAND &amp; GRAVEL, over</div><div>- &gt;6.5m: stiff grey and brown CLAY</div></div></div></div><div>Our Investigation Found:</div><table><thead><tr><th rowspan="2">Strata</th><th colspan="2">Depth Encountered (mBGL)</th><th rowspan="2">Description &amp; Comments</th></tr><tr><th>Top</th><th>Bottom</th></tr></thead><tbody><tr><td>MG: sand &amp; gravel</td><td>0</td><td>1.0, 1.6</td><td>Slightly silty occasionally slightly clayey</td></tr><tr><td>SAND &amp; GRAVEL</td><td>1.0, 1.6</td><td>&gt;3</td><td>Slightly silty. Gravel is flint.</td></tr></tbody></table><div><div>• Anthropogenic components of the made ground comprised concrete and brick, plus rare fragments of plastic, tarmac and wood. There was a pocket of ash in TP2.</div><div>• There were no visual or olfactory indications of contamination noted in the soils during the site works</div></div></div></div></div>			Strata	Depth Encountered (mBGL)		Description & Comments	Top	Bottom	MG: sand & gravel	0	1.0, 1.6	Slightly silty occasionally slightly clayey	SAND & GRAVEL	1.0, 1.6	>3	Slightly silty. Gravel is flint.
Strata	Depth Encountered (mBGL)		Description & Comments														
	Top	Bottom															
MG: sand & gravel	0	1.0, 1.6	Slightly silty occasionally slightly clayey														
SAND & GRAVEL	1.0, 1.6	>3	Slightly silty. Gravel is flint.														
Hydrogeology & Hydrology	<div><div>• <b>Aquifers, Source Protection Zones (SPZ), Abstractions:</b> Not currently checked as unlikely to be significantly affected by the current site.</div><div>• <b>Groundwater (GW):</b> Expected to lie at 0.5m depth. We found no water in our 3m deep pits.</div></div>																
Excavations & Drainage	<div><div><b>Excavations</b><div><div>• Should be possible to &gt;3m depth with conventional earthmoving plant.</div><div>• All of our trial pits remained stable and open during the short time of their formation. There were some collapses during filling of the pits with water, and during the soakage tests.</div><div>• It is unlikely that shallow excavations (&lt;3m) will encounter significant groundwater.</div></div></div><div><b>Drainage</b><div><div>• The slightly silty SAND &amp; GRAVEL gave an infiltration rate of at least 1.25x10<sup>-5</sup>m/s.</div></div></div></div>																

***This is only a summary and should not be read in isolation from the main text.***

## 1 Introduction

### 1.1 Introduction and Brief

AG Geo-Consultants Ltd (AGGC) were commissioned by and on behalf of White House Design (the Client) to undertake soakaway testing at a site known as Land to Rear of The Hut Pub, Old Orchard Cl, Uxbridge, UB8 3LH (the "Site", see plan in Appendix A).

The client's brief was to:

- *Undertake soakaway testing on site.*

### 1.2 Proposed Development

The proposed development (see location plan in Appendix A) comprises a detached house.

### 1.3 Scope of Works

The client accepted AGGC's proposed detailed scope of work for soakaway testing

The client declined intrusive investigation to check any environmental/contamination and geotechnical risks on site.

The objectives of the work were to determine the sub-surface conditions in respect of soakage potential.

### 1.4 Limitations

**Until all invoices associated with the production of this report have been paid in full, then it remains the property of AGGC and not the client, and AGGC do not grant legal reliance upon it to satisfy (or remove) planning permission conditions, or to be used for engineering design, etc.**

**This report is provided for the benefit only of the party to whom it is addressed and their advisors. No other developer or party may use it without our express written permission (i.e. reassignment). We do not accept responsibility to any other third party for the whole or any part of the contents and we exercise no duty of care in relation to this report to any third party.**

Where intrusive investigations have been completed, information, comments and opinions given in this report are based on the ground conditions encountered during the site work and on the results of laboratory and field tests performed during the investigation. However, subsoils are inherently variable and hidden from view such that no investigation can be exhaustive to the extent that all soil conditions are revealed. Conditions may therefore be present beneath the site that were not apparent in the data reviewed as part of this assessment. In particular, it should be noted that groundwater levels vary due to seasonal and other effects, and may at times differ to those measured during the investigation.

This assessment has been based to some extent on data acquired from Third Parties. This data has been accepted as correct and has not been subjected to any additional validation.

Unless specifically noted to the contrary, it should be assumed that this report has not been submitted to any other regulatory authorities for approval. Redevelopment sites in particular

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Soakaway Testing

Land to Rear of The Hut Pub, Old Orchard Cl, Uxbridge

may have planning conditions attached in respect of contaminated land assessment. Apart from the usual generic contaminated land planning conditions, **there can occasionally be site-specific contamination and geotechnical conditions**. Where we are made aware of such conditions in advance of scoping the works, we can tailor the report to the regulatory authority requirements. Where we are not made aware of any such requirements there can be no certainty that our investigation will meet any or all of the regulatory authority requirements.

## 2 Phase 1 Desk Study

### 2.1 Introduction

The following research has been undertaken in order to aid accurate design of the site works.

### 2.2 Desk Study

Table 2.1: Desk Study

Planning conditions	There are no (contaminated land) planning conditions for this site yet, but <b>the NPPF reminds developers (CI 184) that they are responsible for providing developments that are free of significant risks</b> (both contamination and geotechnical). Furthermore, there are ground assessment requirements within <b>Building Regulations</b> . Potential risks to groundworkers should also be considered.
Current Use of Site and Surroundings	<b>On Site Conditions</b> ½ (west) pub beer garden and ½ (east) storage and waste ground area. <b>In the Surroundings</b> Small farm, country park, residential areas.
Historical Land Uses	Not checked
Aerial Photographs	Show nothing extra of significance.
Anticipated Ground Conditions	<b>BGS Mapping Suggests:</b> <ul style="list-style-type: none"> <li>• <b>Fault Lines:</b> None lie significantly close enough to the site.</li> <li>• <b>Made ground (MG):</b> None &gt;1m thickness.</li> <li>• <b>Drift Deposits:</b> Boyn Hill Gravel Member - Sand and gravel.</li> <li>• <b>Solid Geology:</b> London Clay Formation - Clay, silt and sand.</li> <li>• <b>The nearest relevant BGS boreholes (on the same geology) suggest:</b> <ul style="list-style-type: none"> <li>- 0m-6.5m: very dense SAND &amp; GRAVEL, over</li> <li>- &gt;6.5m: stiff grey and brown CLAY</li> </ul> </li> </ul>
Hydrology and Hydrogeology	<b>Watercourses, Aquifers, Source Protection Zones (SPZ), Abstractions:</b> Not checked as unlikely to be significantly affected by the current site. <b>Groundwater (GW):</b> expected to lie at 0.5m depth.
Landfills?	There is a licensed landfill ~95m S of the site.
Potential Ground Risks for Drainage	<ul style="list-style-type: none"> <li>• Possible shallow groundwater</li> <li>• Possible unstable excavations</li> </ul>

## 3 Phase 2 Site Investigation

### 3.1 General

Soakaway testing was carried out on 10<sup>th</sup> June 2025 and employed trial pits. The holes are summarised as follows:

**Table 3.1: Exploratory Hole Details**

Exploratory Hole ID	Technique	Hole Depth (mBGL)	Comments & Reasons for Holes
TP1	Mechanical	3m	Soak tests
TP2	Excavator	3m	

We were not permitted to form trial pits in the pub beer garden (the western ½ of the development site).

The client advised that in the east of the site (~2m from the proposed new footings) there was a pond many years ago which is ~0.6m deep and infilled.

A plan showing the exploratory hole locations is presented as Appendix B. Final hole locations are measured or estimated and were not surveyed.

### 3.2 Trial Pitting

2no. trial pits were excavated using a midi excavator. The trial pits were logged by an onsite engineer.

On completion the pits were backfilled with excavated spoil and compacted.

Detailed log sheets for the trial pits are included in Appendix C.

### 3.3 In-Situ Testing

Soakaway tests were undertaken in the following pits in general accordance with recommended practice given in BRE Digest 365. The results are contained in Appendix D.

**Table 3.2: Soakaway Tests**

Pit Reference	Comments
TP1	3no. fillings were achieved
TP2	3no. fillings were achieved



## 4 Ground Conditions

### 4.1 General

The following table provides a summary of the strata encountered in the exploratory holes and the depth to the base of each stratum. MG = man-made ground.

**Table 4.1: Typical Strata**

Strata	Depth Encountered (mBGL)		Description & Comments
	Top	Bottom	
MG: sand & gravel	0	1.0, 1.6	Slightly silty occasionally slightly clayey
SAND & GRAVEL	1.0, 1.6	>3	Slightly silty. Gravel is flint.

Anthropogenic components of the made ground comprised concrete and brick, plus rare fragments of plastic, tarmac and wood. There was a pocket of ash in TP2.

### 4.2 Groundwater

Groundwater observations were as follows:

**Table 4.2: Groundwater Observations**

Exploratory Hole	Depth to Groundwater (mBGL)		
	During site works		Standing Depths Post-site works
TP1	All holes were <b>dry to their bases</b> at these depths, for the short few hours during their formation:	3.0	-
TP2		3.0	-

### 4.3 Excavations

Excavations to  $\geq 3$ m depth should be suitable with conventional earthmoving plant, although pneumatic tools are likely to be required to break out existing foundations and masonry obstructions.

It is unlikely that shallow excavations ( $< 3$ m) will encounter significant groundwater. If they do, then it may not be possible to keep excavations dry by pumping from a conveniently located sump to a nearby sewer.

If this is required, a temporary discharge licence will be required from the water authority.

If the pumping rate is too high and precautions are not taken, then pumping could suck in fines from the surrounding ground and cause settlements.

All of our trial pits remained stable and open during the short time of their formation. There were some collapses during filling of the pits with water, and during the soakage tests. Temporary excavations in the sand & gravel are therefore not expected to stand unsupported in the short term, either vertically, or with steep cut gradients, and therefore may require shoring or to be battered back to a safe angle of repose. The presence of groundwater will increase excavation side instability. Excavations below approximately 1m depth will require sheeting and shoring for

# AG Geo-Consultants Ltd

Soakaway Testing

Land to Rear of The Hut Pub, Old Orchard Cl, Uxbridge

personnel to enter safely. The stability of all excavations could deteriorate on wetting either from groundwater or surface water. Excavations could therefore be protected from rain and surface water runoff.

## **4.4 Contamination Indications**

There were no visual or olfactory indications of contamination noted in the soils during the site works.

## 5 Soakage Findings

Soakaway infiltration was undertaken in 2no. trial pits. The results are contained in Appendix D and are summarised as follows:

**Table 5.1: Soakaway Results**

Trial Pit	Test Depth range (mbegl)	Corresponding Stratum	Soil Infiltration Rate (m/s)
TP1	1.5-2.45 (~1.0m head)	Slightly silty SAND & GRAVEL	1.25E-05
TP2	1.55-2.5 (~1.0m head)	Slightly silty SAND & GRAVEL	1.91E-05

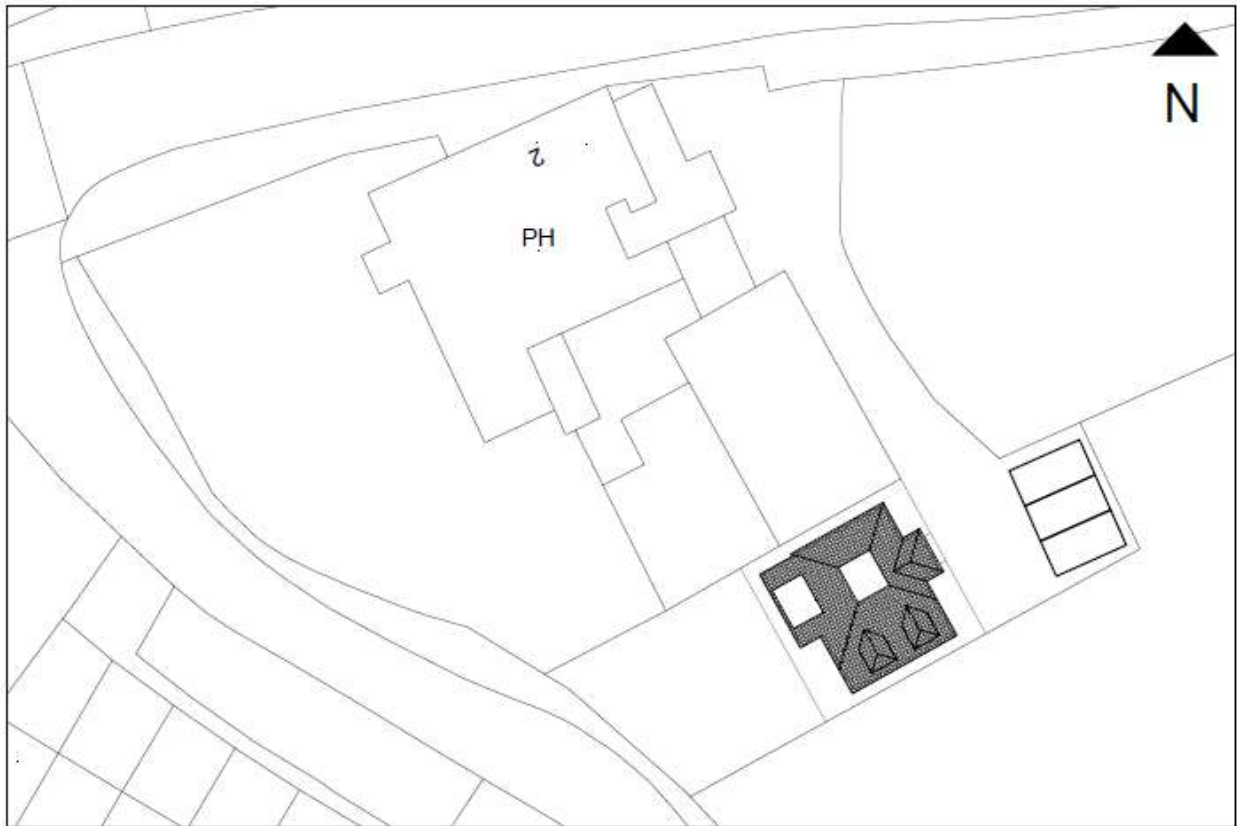
These values (factored in accordance CIRIA 156 (1996) Infiltration Drainage – Manual of Good Practice) may be used for design of soakaways in accordance with BRE Digest 365

## **Appendices**

- A. Proposed Development Plan
- B. Exploratory Hole Locations
- C. Exploratory Hole Logs
- D. In-situ Test Results: Soakaways

# AG Geo-Consultants Ltd

## **Appendix A**

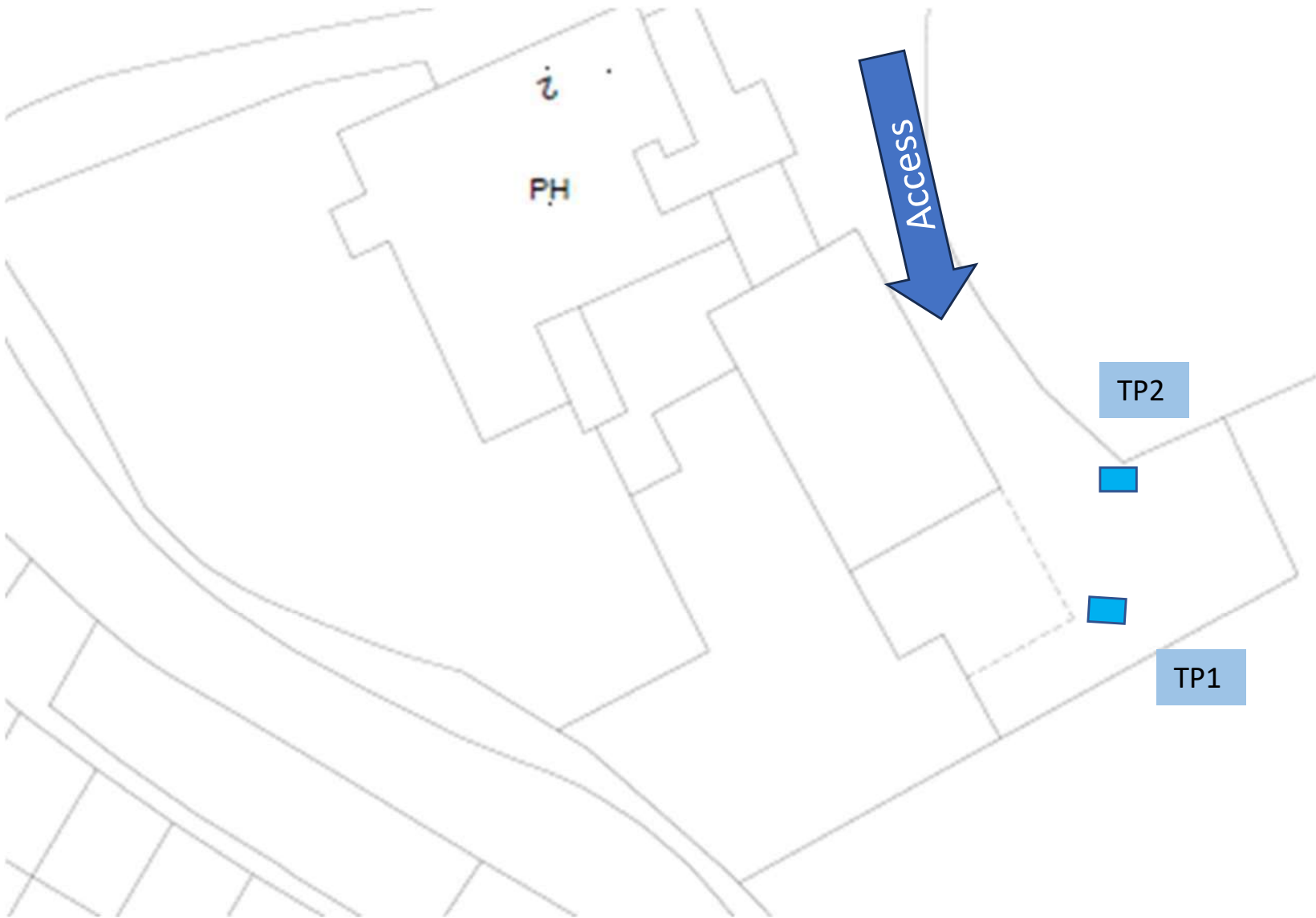


PROPOSED SITE PLAN

# AG Geo-Consultants Ltd

## **Appendix B**

Exploratory Hole  
Location Plan



<u>Version</u>	<u>Date</u>
0	28/05/25
1	08/06/25
2	12/06/25



# AG Geo-Consultants Ltd

## Appendix C

# Key to Exploratory Hole Symbols and Abbreviations

## SAMPLE TYPES

B	Bulk disturbed sample	ES	Environmental soil sample	U	Undisturbed sample
C	Core sample	EW	Environmental water sample	UT	Undisturbed thin wall sample
CBR-D	Disturbed sample from CBR test area	G	Gas sample	W	Water sample
CBR-U	Undisturbed sample from CBR test area	L	Liner sample		
D	Small disturbed sample	SPT	SPT split spoon sample		

## IN-SITU TESTING

SPTs	Standard Penetration Test (using a split spoon sampler)
SPTc	Standard Penetration Test (using a solid 60 degree cone)
N	Recorded SPT 'N' Value *
-/-	Blows/Penetration (mm) after seating blows totalling 150 mm
MX	Mexi Probe Test (records CBR as %)
HV	Hand Shear Vane Test (undrained shear strength quoted in kPa)
HP	Hand Penetrometer Test (kg/m <sup>3</sup> )
( )	Denotes residual test value
PID	Photo Ionisation Detector (ppm) *
Kf/Kr	Permeability Test (f = falling head, r = rising head quoted in ms <sup>-1</sup> )
HPD	High Pressure Dilatometer Test (pressure meter)
PKR	Packer / Lugeon Permeability Test
CBR	California Bearing Ratio Test

## ROTARY CORE DETAILS

TCR	Total Core Recovery, %
SCR	Solid Core Recovery, %
RQD	Rock Quality Designation (% of intact core >100 mm)
FI	Fracture Spacing (average fracture spacing; in mm, over indicated length of core) **
NI	Non-Intact Core
AZCL	Assumed Zone of Core Loss

## GROUNDWATER



Groundwater strike

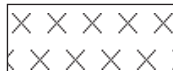


Standing water level after 20 minutes; 1st, 2nd etc (number denotes level order)

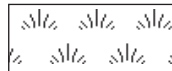
## STRATA LEGENDS - Note: Composite strata types are shown by combining symbols



Made Ground



Silt



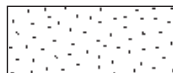
Peat



Limestone



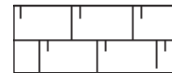
Concrete



Sand



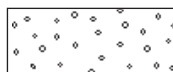
Void



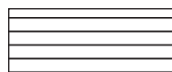
Chalk



Bituminous  
Bound Materials



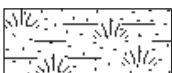
Gravel



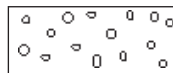
Mudstone



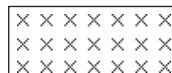
Coal



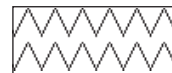
Topsoil



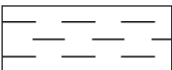
Cobbles



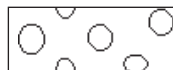
Siltstone



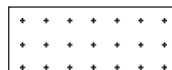
Metamorphic Rock



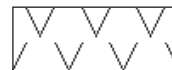
Clay



Boulders



Sandstone



Fine Grained Igneous  
Rock

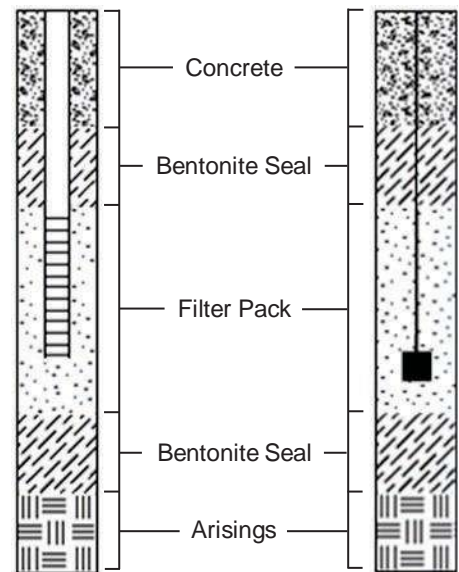
\* Where a single value is quoted this is the uncorrected 'N' value for a full 300 mm test drive following a seating drive of 150mm. Where the full test drive penetration is not achieved the number of blows is quoted for the penetration below the test total of 300mm, e.g.: 50/75.

\*\* The minimum, average and maximum are shown e.g. 5/45/125.

## INSTALLATION & BACKFILL DETAILS

Standpipe

Piezometer



Perforated  
Standpipe



Plain  
Standpipe



Piezometer  
Porous Element

## STRATUM BOUNDARIES

Unit boundary

<div style="border: 1px solid black; padding: 2px; margin-bottom: 5px;"> <b>THAMESIDE</b>  <b>GROUNDWORKS</b> </div>	TP Log
Site:     The Hut Pub, Uxbridge	Job No: 24-045
Client:	Date: 10/06/2025
Engineer:   AG Geo	Hole No: TP1

STRATA														
TOP DEPT H (m)	DESCRIPTION	SAMPLES			TESTS								TOP DEPT H (m)	
		TYPE	DEPTH (m)	No. 0.5kg tub or bulk bags	HP (6mm)			HSV (19mm)						
					Kg/cm2		kN/ m2	1	2	3	Av.			
0.0	MADE GROUND. Brown slightly silty very gravelly fine to medium sand. Gravel is fine to coarse angular to rounded of flint. Frequent fragments and cobbles of brick and concrete.													0.0
0.1														0.1
0.2														0.2
0.3	MADE GROUND. Orange and red slightly silty very sandy gravel. Gravel fine to coarse of flint. Abundant fragments and cobbles of brick. Rare plastic sheet.													0.3
0.4														0.4
0.5														0.5
0.6	MADE GROUND. Brown and grey brown slightly silty slightly clayey grvaely sand. Gravel is fine to coarse angular to rounded of flint. Sand is medium to coarse. Occasional frgment of brick.													0.6
0.7														0.7
0.8														0.8
0.9														0.9
1.0	Orangish brown slightly silty GRAVEL AND SAND. Gravel is fine to coarse angular to rounded of flint. Sand is coarse.													1.0
1.1														1.1
1.2														1.2
1.3														1.3
1.4														1.4
1.5														1.5
1.6														1.6
1.7														1.7
1.8														1.8
1.9														1.9
2.0														2.0
2.1														2.1
2.2														2.2
2.3														2.3
2.4														2.4
2.5														2.5
2.6														2.6
2.7														2.7
2.8														2.8
2.9														2.9
3.0														3.0

<b>Comments:</b>	W3W ///fame.others.cities
	No visible signs of contamination or obvious sources of contamination nearby.
	Site area generally littered with assorted building debris, equipment etc.
Groundwater:	None
Pitwall Stability:	Stable during excavation. Some collapses during soakage fill/test
Dimensions:	1.3 x 0.35 x 3.0 (reduced to 2.4m during soakage)
Backfill:	Arising

<div style="display: flex; align-items: center;"> </div>	TP Log
Site:     The Hut Pub, Uxbridge	Job No. 24-045
Client:	Date: 10/06/2025
Engineer:   AG Geo	Hole No: TP2

STRATA													
TOP DEPT H (m)	DESCRIPTION	SAMPLES			TESTS								TOP DEPT H (m)
		TYPE	DEPTH (m)	No. 0.5kg tub or bulk bags	HP (6mm)				HSV (19mm)				
					Kg/cm2		kN/ m2	1	2	3	Av.		
0.0	MADE GROUND. Brown slightly silty very gravelly fine to medium sand. Gravel is fine to coarse angular to rounded of flint. Frequent fragments and cobbles of brick and concrete. Occasional fragments of tarmac, wood, plastic. Pocket of ash at 0.5m												0.0
0.1													0.1
0.2													0.2
0.3													0.3
0.4													0.4
0.5													0.5
0.6													0.6
0.7													0.7
0.8													0.8
0.9													0.9
1.0													1.0
1.1													1.1
1.2													1.2
1.3													1.3
1.4													1.4
1.5													1.5
1.6													1.6
1.7													1.7
1.8													1.8
1.9													1.9
2.0	Orangish brown slightly silty GRAVEL AND SAND. Gravel is fine to coarse angular to rounded of flint. Sand is coarse.												2.0
2.1													2.1
2.2													2.2
2.3													2.3
2.4													2.4
2.5													2.5
2.6													2.6
2.7													2.7
2.8													2.8
2.9													2.9
3.0													3.0

<b>Comments:</b>	W3W ///moss.books.activism
	No visible signs of contamination or obvious sources of contamination nearby.
	Site area generally littered with assorted building debris, equipment etc.
Groundwater:	None
Pitwall Stability:	Stable during excavation. Some collapses during soakage fill/test
Dimensions:	1.4 x 0.35 x 3.0 (reduced to 2.5m during soakage)
Backfill:	Arising

# AG Geo-Consultants Ltd

## **Appendix D**

Site: The Hut Pub, Uxbridge

Client:

Engineer: AG Geo

Job Number
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24-045

Sheet:

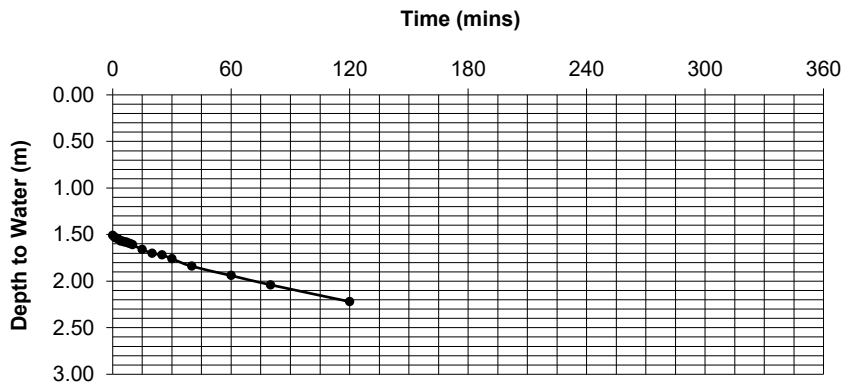
1 / 1

## Soakaway Test

Hole No: **TP1**

TEST NO: 1

DATE: 10/06/25

[illegible]

Length of pit:  $L = 1.30$  m

Width of pit:  $W = 0.35$  m

Depth of pit       $D = 2.45$  m

Base area of pit:  $A = 0.46 \text{ m}^2$

100% effective depth      D100 =      1.51      m

75% effective depth      D75 =      1.75      m

50% effective depth      D50 =      1.98      m

25% effective depth      D25 =      2.22      m

time to D75      T75 = 1740 sec

time to D25      T25 =      7200      sec

time from D75 to D25       $t_{p75-25} =$       5460      sec

(T25 - T75)

volume between D75 & D25  $V_{p75-25} = 0.21 \text{ m}^3$

$$(A \times (D25 - D75))$$

surface area to D50 inc. base       $a_{p50} = 2.01 \text{ m}^2$

$$((2 \times (D - D_{50}) \times (W + L)) + A)$$

## SOIL INFILTRATION RATE

$$f = \frac{V_{p75-25}}{a_{p50} \times t_{p75-25}}$$

**f = 1.95E-05 m/sec**

Test Strata: GRAVEL AND SAND  
(see Trial Pit)

Remarks: Pitwall stability good during excavation but some collapses during fill

Site: The Hut Pub, Uxbridge

Client:

Engineer: AG Geo

Job Number
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24-045

Sheet:

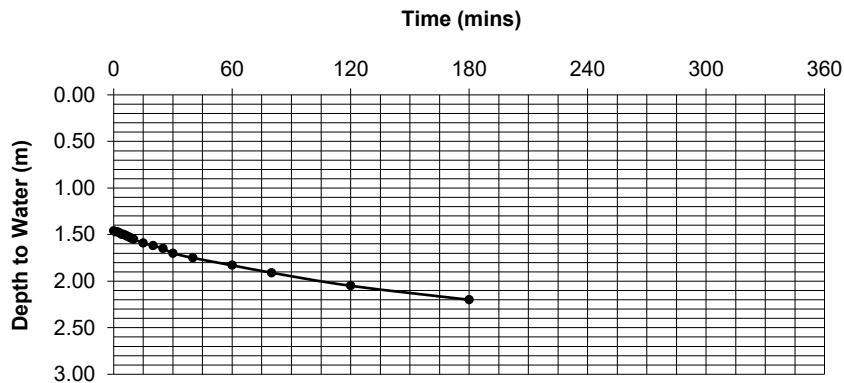
1 / 1

## Soakaway Test

Hole No: **TP1**

TEST NO: 2

DATE: 10/06/25

[illegible]

Length of pit:  $L = 1.30$  m

Width of pit:  $W = 0.35$  m

Depth of pit      D =      2.45      m

Base area of pit:  $A = 0.46 \text{ m}^2$

100% effective depth      D100 =      1.46      m

75% effective depth      D75 =      1.71      m

50% effective depth      D50 =      1.96      m

25% effective depth      D25 =      2.20      m

time to D75      T75 = 1860 sec

time to D25      T25 =      10800      sec

time from D75 to D25       $t_{p75-25} =$       8940      sec

(T25 - T75)

volume between D75 & D25       $V_{p75-25} = 0.23 \text{ m}^3$

$$(A \times (D25 - D75))$$

surface area to D50 inc. base       $a_{p50} = 2.09 \text{ m}^2$

$$((2x(D-D50)x(W+L)) + A)$$

## SOIL INFILTRATION RATE

$$f = \frac{V_{p75-25}}{a_{p50} \times t_{p75-25}}$$

**f = 1.21E-05 m/sec**

Test Strata: GRAVEL AND SAND  
(see Trial Pit)

Remarks: Pitwall stability good during excavation but some collapses during fill

Site: The Hut Pub, Uxbridge

Job Number
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24-045

Client:

Sheet:

Engineer: AG Geo

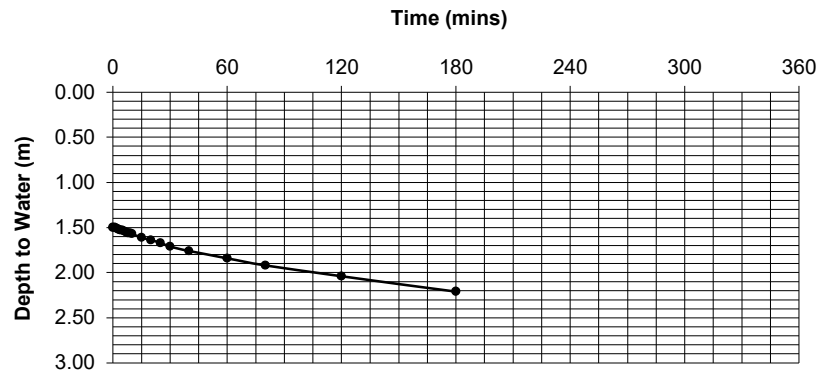
1 / 1

## Soakaway Test

Hole No: TP1

TEST NO: 3

DATE: 10/06/25

[illegible]

Length of pit:	L =	1.30	m
Width of pit:	W =	0.35	m
Depth of pit:	D =	2.45	m
Base area of pit:	A =	0.46	m <sup>2</sup>

100% effective depth	D100 =	1.50	m
75% effective depth	D75 =	1.74	m
50% effective depth	D50 =	1.98	m
25% effective depth	D25 =	2.21	m

time to D75	T75 =	2280	sec
time to D25	T25 =	10800	sec

time from D75 to D25       $t_{p75-25} =$       8520      sec  
(T25 - T75)

volume between D75 & D25  $V_{p75-25} = 0.22 \text{ m}^3$   
( $A \times (D25 - D75)$ )

surface area to D50 inc. base  $a_{p50} = 2.02 \text{ m}^2$   
 $((2 \times (D-D50) \times (W+L)) + A)$

## SOIL INFILTRATION RATE

$$f = \frac{V_{p75-25}}{a_{p50} \times t_{p75-25}}$$

**f = 1.25E-05 m/sec**

Test Strata: GRAVEL AND SAND  
(see Trial Pit)

Remarks: Pitwall stability good during excavation but some collapses during fill  
Terminated after 1 hour due to time constraints  
Blue Text = extrapolated results





Site: The Hut Pub, Uxbridge

Job Number	24-045
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Client:

Sheet:

Engineer: AG Geo

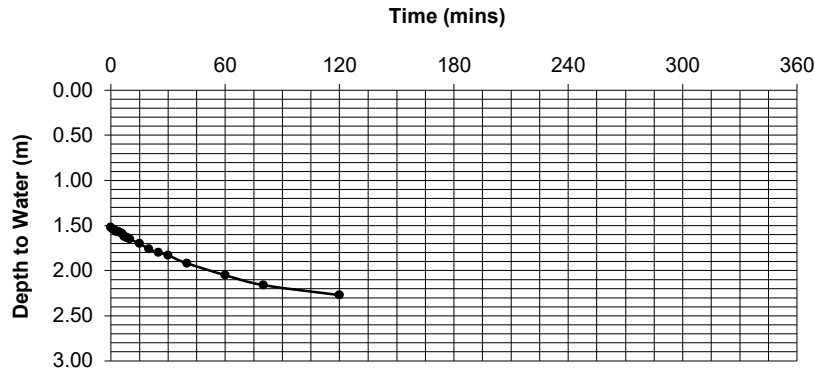
1 / 1

## Soakaway Test

Hole No: **TP2**

TEST NO: 2

DATE: 10/06/25

[illegible]

Length of pit:	L =	1.40	m
Width of pit:	W =	0.35	m
Depth of pit:	D =	2.50	m
Base area of pit:	A =	0.49	m <sup>2</sup>

100% effective depth	D100 =	1.52	m
75% effective depth	D75 =	1.77	m
50% effective depth	D50 =	2.01	m
25% effective depth	D25 =	2.26	m

time to D75	T75 =	1260	sec
time to D25	T25 =	6960	sec

time from D75 to D25       $t_{p75-25} =$       5700      sec  
(T25 - T75)

volume between D75 & D25  $V_{p75-25} = 0.24 \text{ m}^3$   
(A x (D25 - D75))

surface area to D50 inc. base  $a_{p50} = 2.21 \text{ m}^2$   
 $((2 \times (D - D50) \times (W + L)) + A)$

## SOIL INFILTRATION RATE

$$f = \frac{V_{p75-25}}{a_{p50} \times t_{p75-25}}$$

**f = 1.91E-05 m/sec**

Test Strata: GRAVEL AND SAND  
(see Trial Pit)

Remarks: Pitwall stability good during excavation but some collapses during fill

Site: The Hut Pub, Uxbridge

Job Number	24-045
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Client:

Sheet:

Engineer: AG Geo

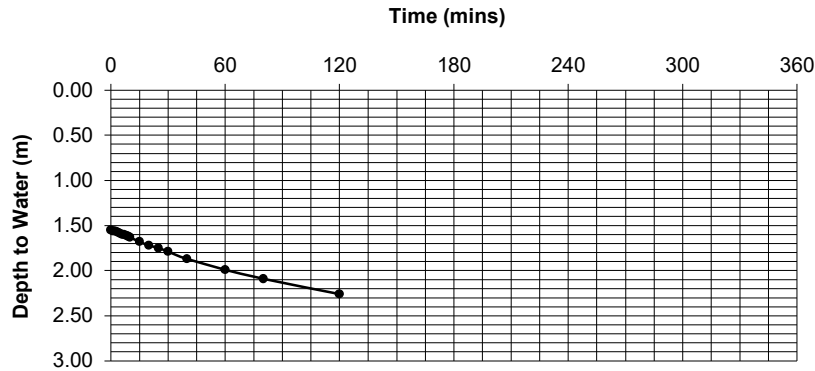
1 / 1

## Soakaway Test

Hole No: **TP2**

TEST NO: 3

DATE: 10/06/25

[illegible]

Length of pit:	L =	1.40	m
Width of pit:	W =	0.35	m
Depth of pit:	D =	2.50	m
Base area of pit:	A =	0.49	m <sup>2</sup>

100% effective depth	D100 =	1.55	m
75% effective depth	D75 =	1.79	m
50% effective depth	D50 =	2.03	m
25% effective depth	D25 =	2.26	m

time to D75	T75 =	1800	sec
time to D25	T25 =	7200	sec

time from D75 to D25       $t_{p75-25} =$       5400      sec  
(T25 - T75)

volume between D75 & D25  $V_{p75-25} = 0.23 \text{ m}^3$   
(A x (D25 - D75))

surface area to D50 inc. base  $a_{p50} = 2.15 \text{ m}^2$   
 $((2 \times (D - D50) \times (W + L)) + A)$

## SOIL INFILTRATION RATE

$$f = \frac{V_{p75-25}}{a_{p50} \times t_{p75-25}}$$

**f = 2.00E-05 m/sec**

Test Strata: GRAVEL AND SAND  
(see Trial Pit)

Remarks: Pitwall stability good during excavation but some collapses during fill  
Terminated at 80 minutes due to time constraints  
Blue Text = extrapolated results