## SuDS Flows and Volumes – Hillingdon LLFA Technical Assessment Proforma

Hillingdon LLFA Technical Assessment Proforma Revision 2 – Issued January 2018

2.15	Design level used for surcharge water level at point of discharge <sup>(14)</sup>		
2.16	Provide outfall levels if discharging to a watercourse		
CALCULATION OUTPUTS REQUIRED			
Note: Sections 3 and 4 refer to site where storage is provided by attenuation and / or partial infiltration. Where all flows are infiltrated to ground omit Sections 3 -5 and complete Section 6.			
3.0 Defining rate of runoff from the site. These values must be calculated for each return period and should vary.			
3.2	Max. discharge for 1 in 1 year rainfall3.5		
3.2	Max. discharge for Q <sub>med</sub> rainfall		
3.3	Max. discharge for 1 in 30 year rainfall		
3.4	Max. discharge for 1 in 100 year rainfall		
3.5	Max. discharge for 1 in 100 year plus 40%CC		
4.0 Attenuation storage to manage peak runoff rates from the site. These values must be calculated for each return period			
4.1	Storage - 1 in 1 year U.S. m³ O.0033 (of developed impermeable area)		
4.2	Storage - 1in 30 year (7) Q		
4.3	Storage - 1in 100 year (8)		
4.4	Storage - 1 in 100 year plus 40%CC (9) 2.38 m³ (2.0.1)2m³/m²		
5.0 Controlling volume of runoff from the site			
5.1	Pre development runoff volume <sup>(10)</sup> 54.38 m³ for the site		
5.2	Post development runoff volume (unmitigated) (10) 5.7.29 m³ for the site		
5.3	Volume to be controlled/does not leave site (5.2-5.1)291 m³ for the site  Volume control provided by:		
5.4	1. Interception losses <sup>(11)</sup> 2. Rain harvesting <sup>(12)</sup> 3. Infiltration (even at very low rates) 4. Separate area designated as long term storage <sup>(13)</sup> 1. Interception losses <sup>(11)</sup> 1		
5.5	Total volume control (sum of inputs for 5.4 (1-4)		
6.0 Site storage volumes (full infiltration only)			
6.1	Storage - 1in 30 year <sup>(7)</sup> m <sup>3</sup> m <sup>3</sup> /m <sup>2</sup> (of developed impermeable area)		
6.2	Storage - 1 in 100 year plus CC (9)m <sup>3</sup> m <sup>3</sup> /m <sup>2</sup>		

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This form identifies the information required by the LLFA to enable technical assessment of flows and volumes determined as part of drainage / SuDS calculations.

Note: \* means delete as appropriate; Numbers in brackets refer to accompanying notes.

## SITE BACKGROUND INFORMATION

1.0 SITE DETAILS AND RELEVANT CLASSIFICIATIONS				
1.1	Planning application reference	NET YET SUBMITTED		
1.2	Site name	VICTORIA RETAIL PARK		
1.3	Total application site area (1)	1700 m <sup>2</sup> 017 ha		
1.4	Is the site located in a area at SW flood risk, a CDA or LFRZ  (see L.B. Hillingdon website SWMP)  Is the site located in a SPZ  (refer to Environment Agency website Groundwater)			
1.5				
2.0 VOLUME AND FLOW DESIGN INPUTS				
2.1	Site area which is positively drained by S	SuDS <sup>(2)</sup> m <sup>2</sup>		
2.2	Impermeable area drained pre developm	nent (3) \$15m <sup>2</sup>		
2.3	Impermeable area drained post develop	ment (3) 1389 m <sup>2</sup>		
2.4	Additional impermeable area (2.3 minu	s 2.2)5.65m <sup>2</sup>		
2.5	Predevelopment use (4)	Greenfield / Brownfield / Mixed*		
2.6	Method of discharge (5)	Infiltration (waterbody / storm sewer / combined sewer*		
2.7	Infiltration rate (evidence to be supplied	to BRE365 as minimum)m/s		
2.8	Influencing factors on infiltration	LOWDON CLAY		
2.9	Depth to highest known ground water tal	blemAOD		
2.10	Coefficient of runoff (Cv) (6)	l o		
2.11	The LLFA would typically anticipate a Cv of 0.95 from roof These values should be applied for both summer and wint Justification for Cv used if less than this			
2.12	FEH rainfall data used (for Hillingdon typ	rical value 4l/s/ha (Note that FSR not to be used))		
2.13	Will storage be subject to surcharge by	elevated water levels in watercourse / sewer Y N		
2.14	Invert level at outlet (invert level of final f	low control) 37.39.2 mAOD		

2.14 Invert level at outlet (invert level of final flow control)